

**BEARING NOTE:**

BEARINGS ARE ASTRONOMIC AND DERIVED FROM THE NORTH WEST LIMIT OF MULLIGAN STREET AS SHOWN ON PLAN 496 BEING N 59° 37' E.

Note: Not For Registration Purposes

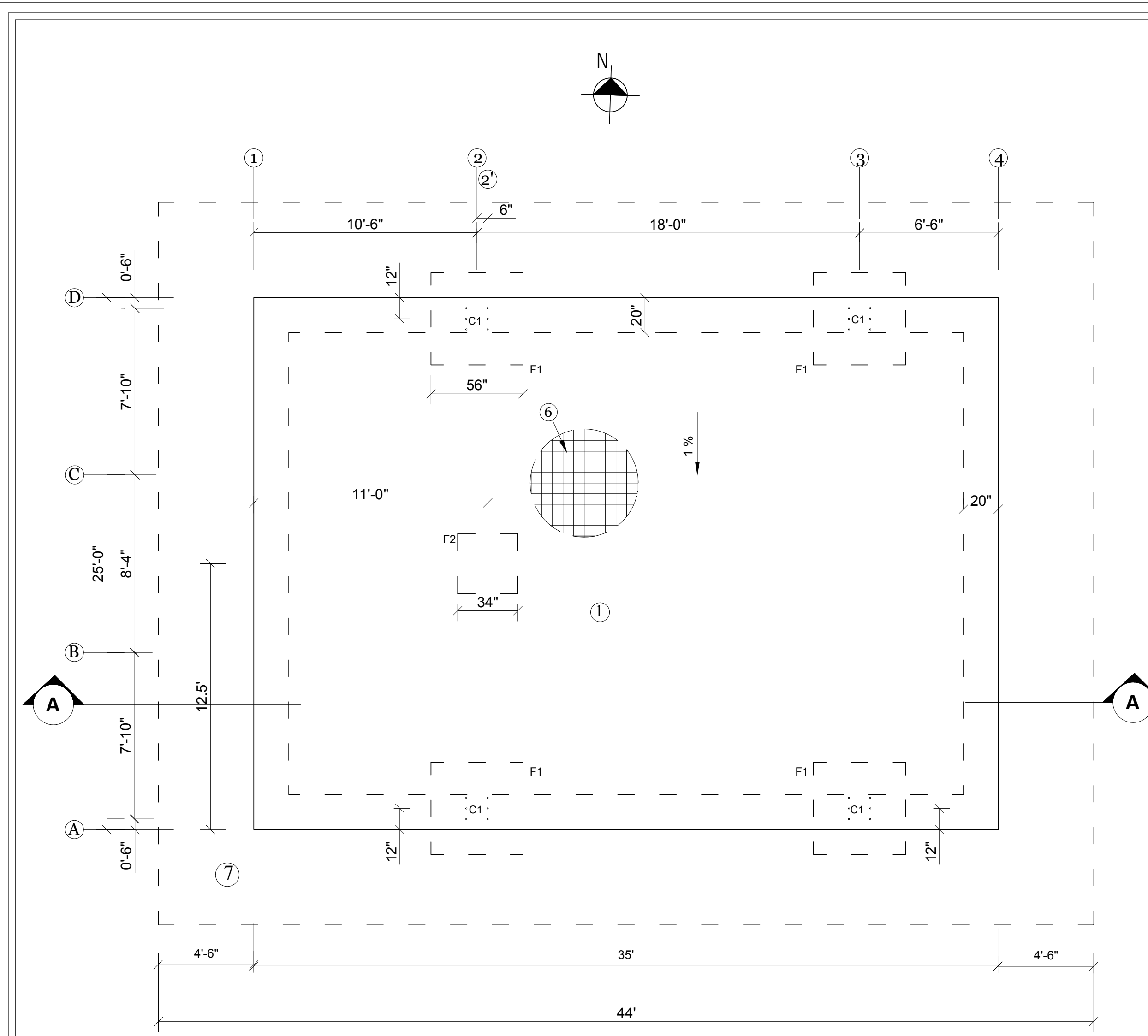
LEGEND (If Applicable)	
SIB - STANDARD IRON BAR	□ - FOUND
IB - IRON BAR	○ - PLANTED
⊕ - ROUND IRON BAR	CC - CUT CROSS

**FARLEY & MARTIN LTD.**  
 Professional Engineers — Ontario Land Surveyors  
 OTTAWA, ONTARIO

**SURVEYOR'S CERTIFICATE**  
 I CERTIFY THAT  
 THE FIELD SURVEY REPRESENTED ON THIS PLAN WAS  
 COMPLETED ON THE 4 DAY OF AUGUST 1983  
 AUGUST 5 1983  
 DATE  
 H. R. Farley  
 H. R. FARLEY  
 ONTARIO LAND SURVEYOR

**PART OF LOT 15  
 CONCESSION I RIDEAU FRONT  
 TOWNSHIP OF GLOUCESTER**

Scale: 1" = 40'  
 Revised:

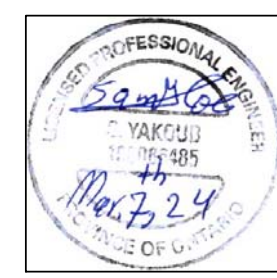


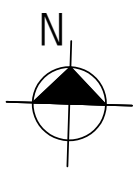
Slab on grade, plan  
Scale : 1/50

Single Footings Table					
Name	Dimensions		Cross Reinf. footings	Long Reinf. footings	Notes
	L x W	Thickness			
F1	56" x 56"	14"	#15M @ 10" O.C.	#15M @ 10" O.C.	Reinforced concrete, 7 % air entrained, 32 MPa, clear cover 2".
F2	34" x 34"	8"	#15M @ 10" O.C.	#15M @ 10" O.C.	. . .

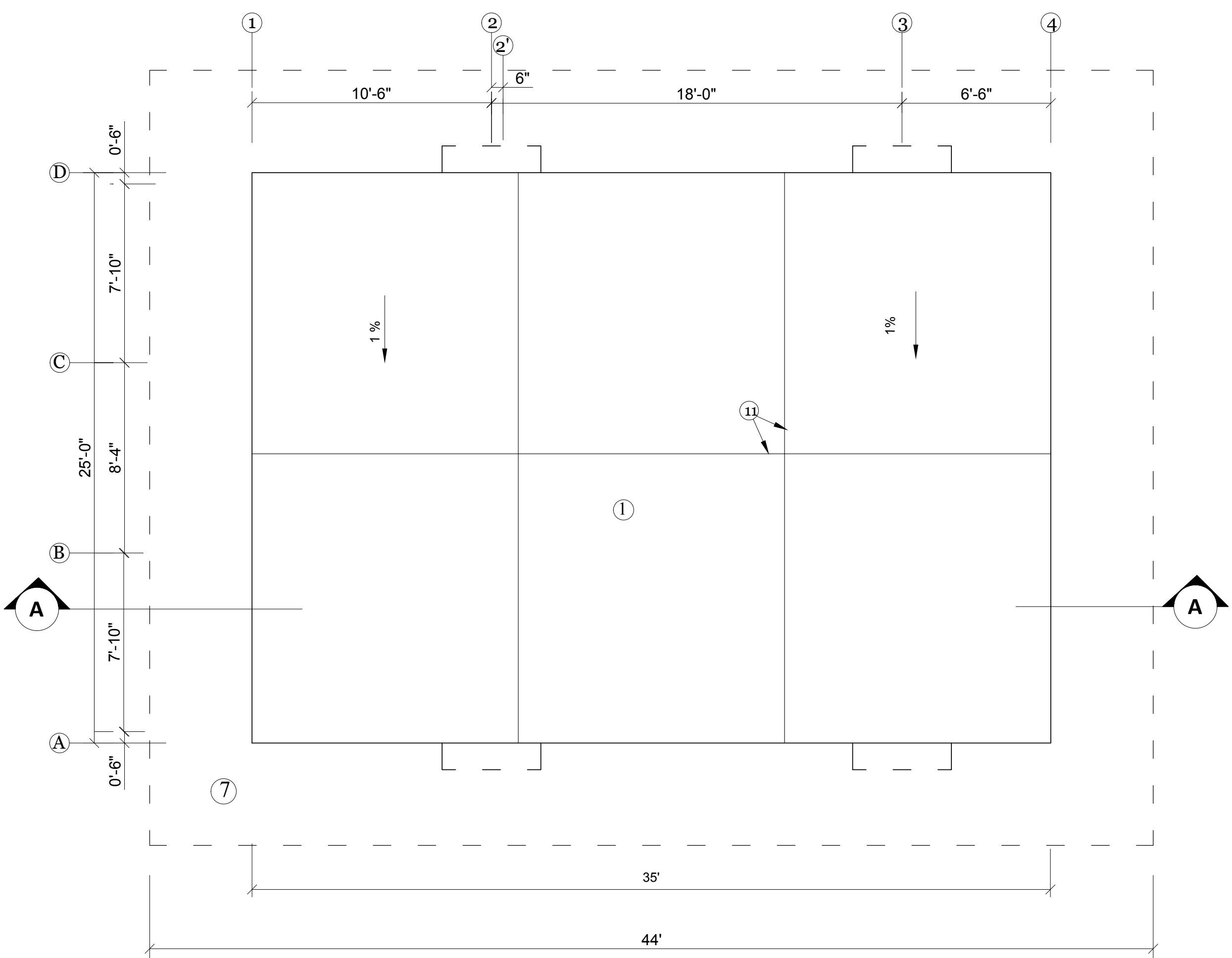
Short column Table				
Name	Size	Long. Reinf./Dowels	Stirrups	Notes
C1	14" x 14"	6 # 15M	-	Reinforced concrete, 7 % air entrained, 32 MPa.

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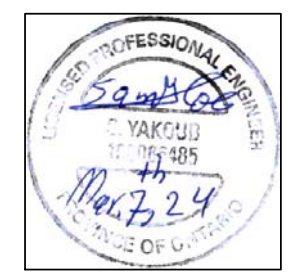




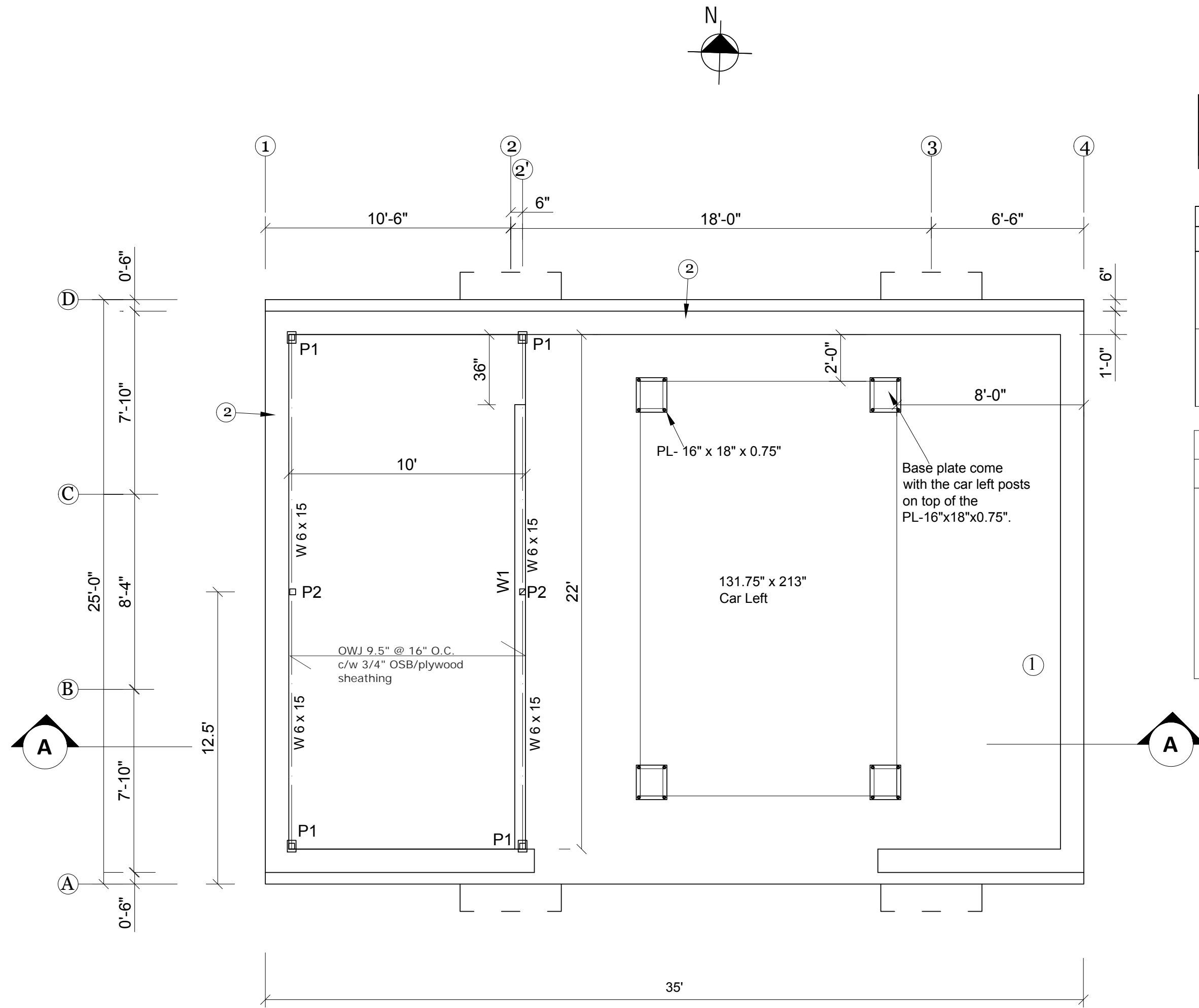
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Slab on grade, Contraction joints, Plan  
Scale :1/50



Clint Millar-Langlois 1419 Mulligan St. Gloucester ON K1V 1H4, terrydaley1@gmail.com	Project A slab on grade for a new garage	Ottawa Structural Engineering Services Inc 200-100 Metcalfe St. Unit 353 Ottawa, ON K1P 5M1	Drawings and Structural Review, Sam Yakoub , Structural Engineer, Ph.D, PEng.	Title Slab on grade, contraction joints	Drawing No <b>S2</b>
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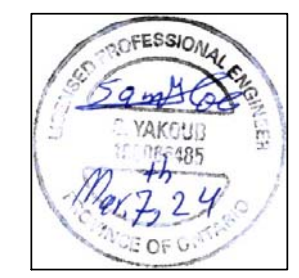
Beam Table		
Name	Size	Notes
B1	W 6 x 15	Steel Beam, 22' long including min 3.5" bearing distance on each end post.

Post Table			
Name	Size	Details	Notes
P1	Adjustable 2.5" dia	Rated for Min 8,000 lbs such as Blackjack from Mitek or equivalent	C / W, 3 gauge 6" x 4.5" top and bottom plates. Bottom steel plate fixed to concrete footing /slab with 4-3/8" x 3.5" Tapcon screws. Top plate fixed to beam bottom flange with 4-3/8" x 1.5" through bolts.
P2	Adjustable 3" dia	Rated for Min 18,000 lbs such as Redjack from Mitek or equivalent	" " "

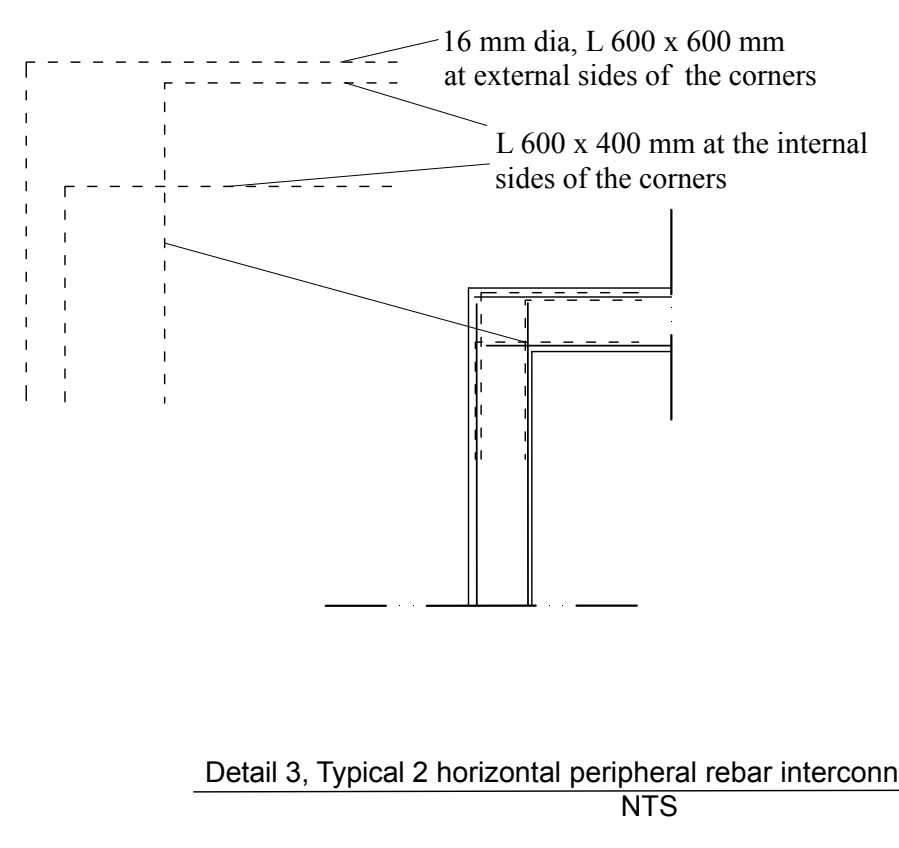
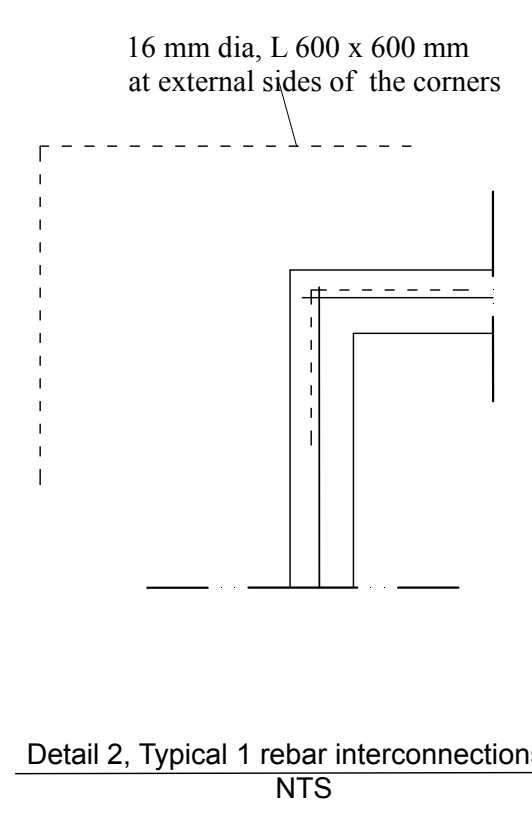
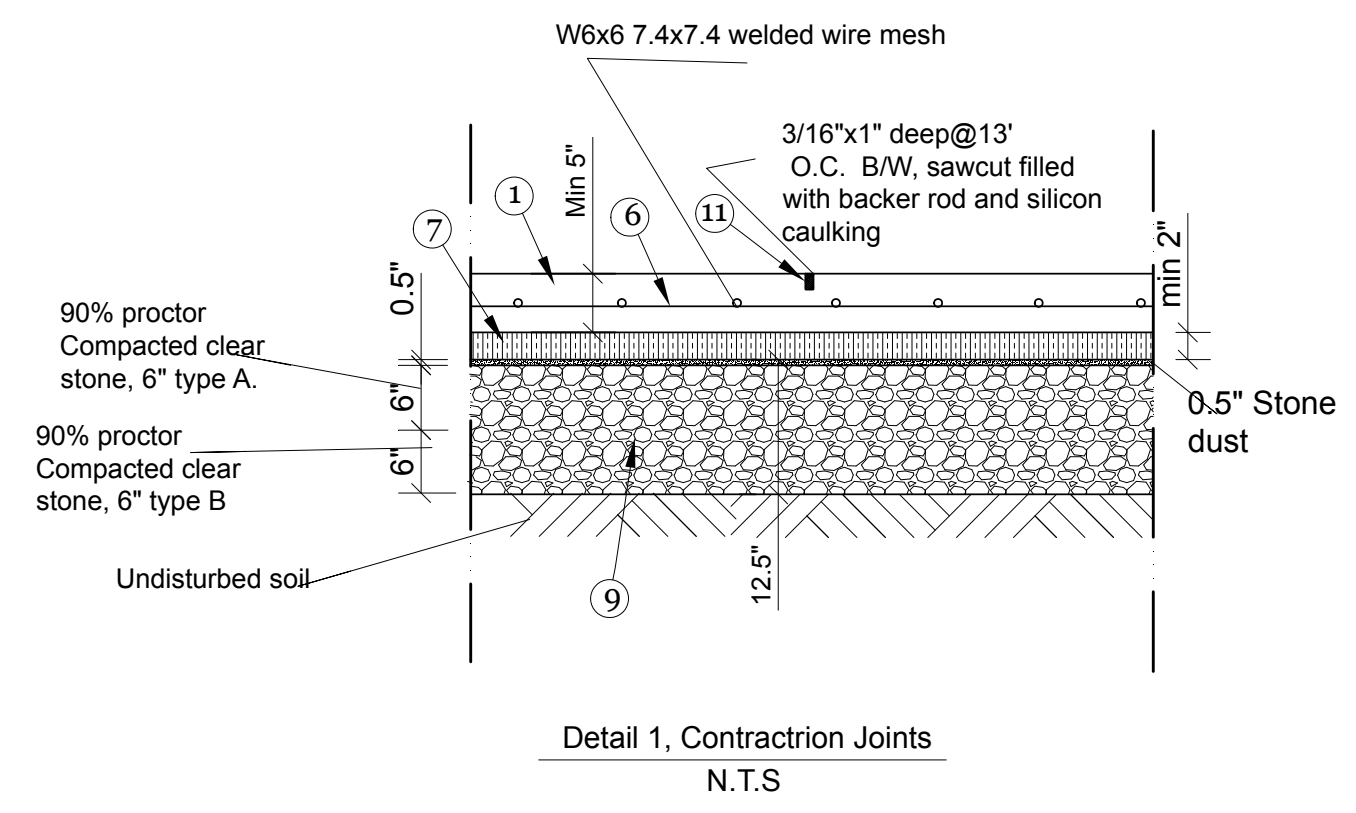
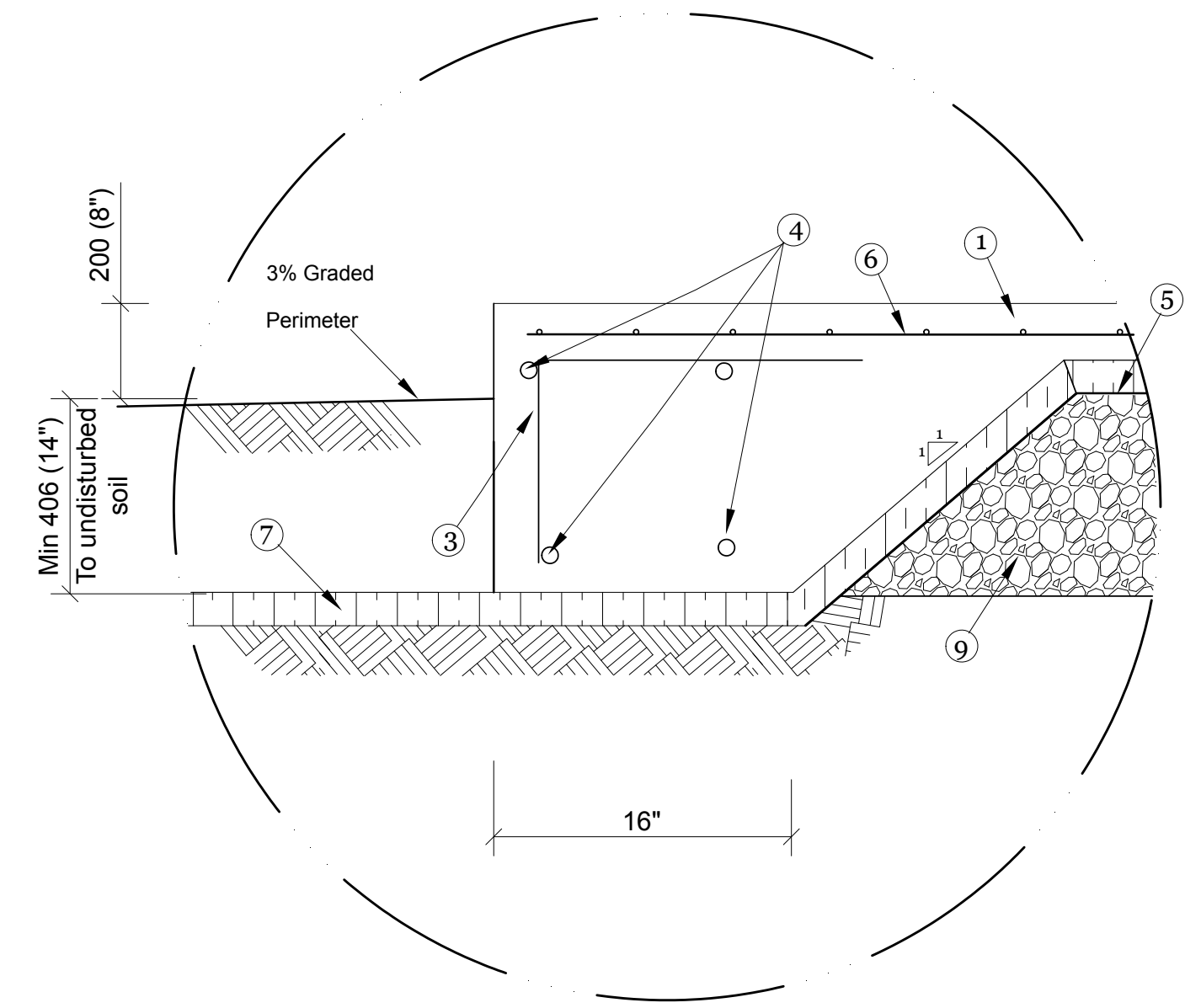
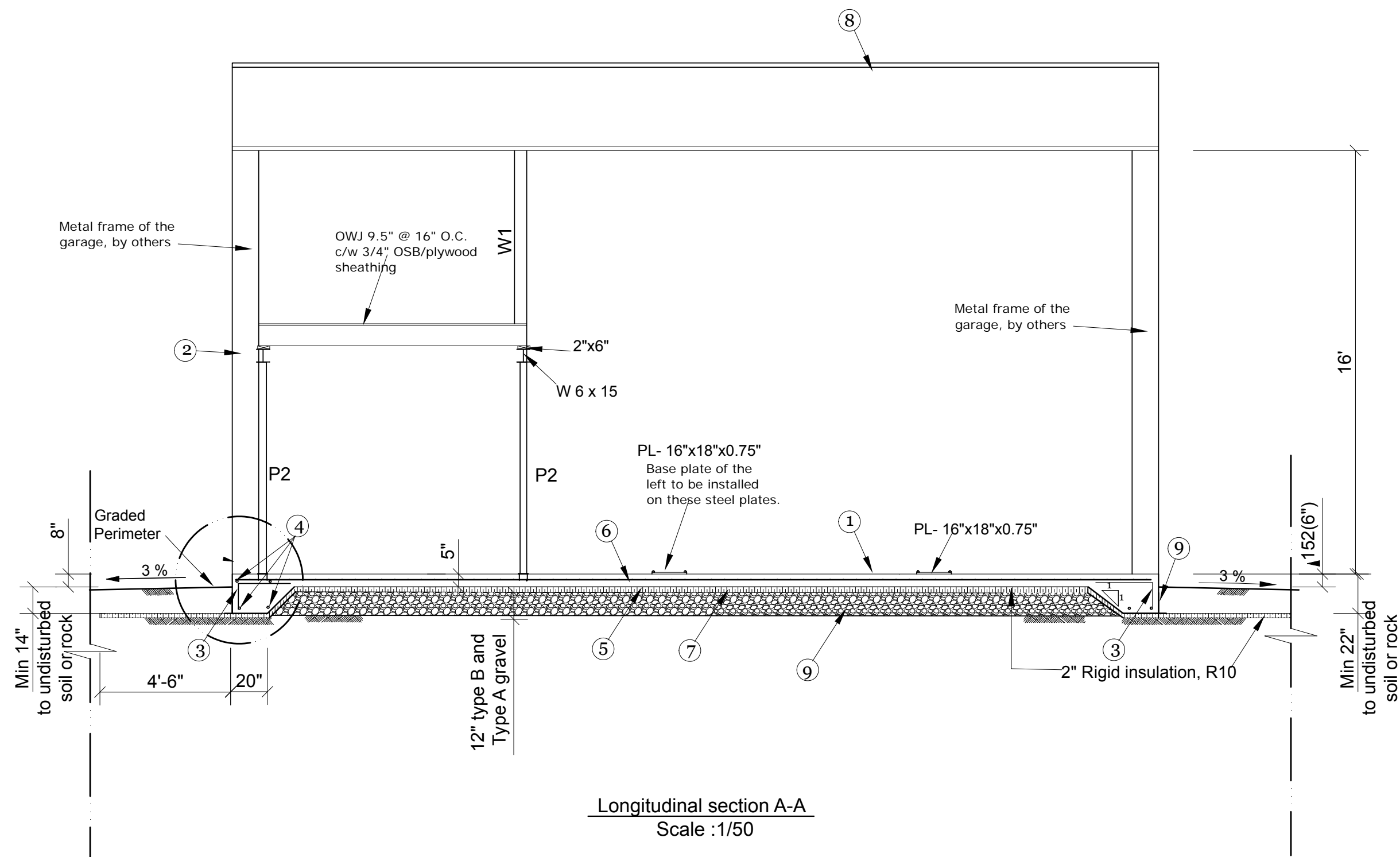
Wall table	
Notes	W1
	Loft
Studs	2" x 6" @ 16" mm O.C. c / w 2" x 6" bottom plate and 2- 2" x 6" top plates. Double studs at adjoining panel edges .
Sheathing	1/2" OSB or plywood, 1 side / outside, and c/w 1/2" drywall on the inside.
Fasteners	76 (3") x 3.66 @ 100 mm (3") O.C. nails on panel edges, 6" O.C. in the middle, or # 8 x 3" @ 3" O.C. screws at panel edges and 6" in middle. For drywall # 6 x 1.5" @ 6" O.C. drywall screws.
Anchors	Anchor bottom plates using 2-1/4" x 2.25" @ 4" O.C., SDS screws staggered, (leave 1" to edges) and fix each stud to bottom plate with min 3-4.5" # 8, toe screws, 2 screws on one side, and a screw on the other side, 2" away from bottom of studs.

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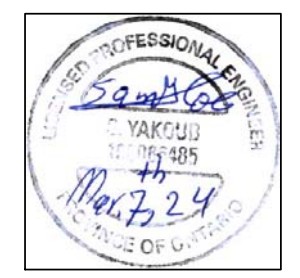
Loft floor and supports, and car left bases , Plan  
Scale : 1/50



Clint Millar-Langlois 1419 Mulligan St. Gloucester ON K1V 1H4, terrydaley1@gmail.com	<b>Project</b> A slab on grade for a new garage	<b>Ottawa Structural Engineering Services Inc</b> 200-100 Metcalfe St. Unit 353 Ottawa, ON K1P 5M1	Drawings and Structural Review, Sam Yakoub , Structural Engineer, Ph.D, PEng.	<b>Title</b> Slab on grade, loft floor and supports, and HDS-14 left bases	<b>Drawing No</b> <span style="font-size: 2em; font-weight: bold;">S3</span>
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Clint Millar-Langlois 1419 Mulligan St. Gloucester ON K1V 1H4, terrydaley1@gmail.com	<b>Project</b> A slab on grade for a new garage	<b>Ottawa Structural Engineering Services Inc</b> 200-100 Metcalfe St. Unit 353 Ottawa, ON K1P 5M1	Drawings and Structural Review, Sam Yakoub, Structural Engineer, Ph.D, PEng.	<b>Title</b> Slab on grade, section and details	<b>Drawing No</b> S4
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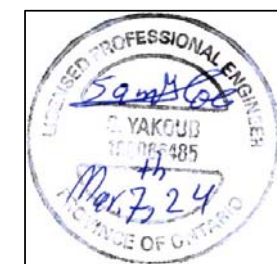
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**Structural Notes**

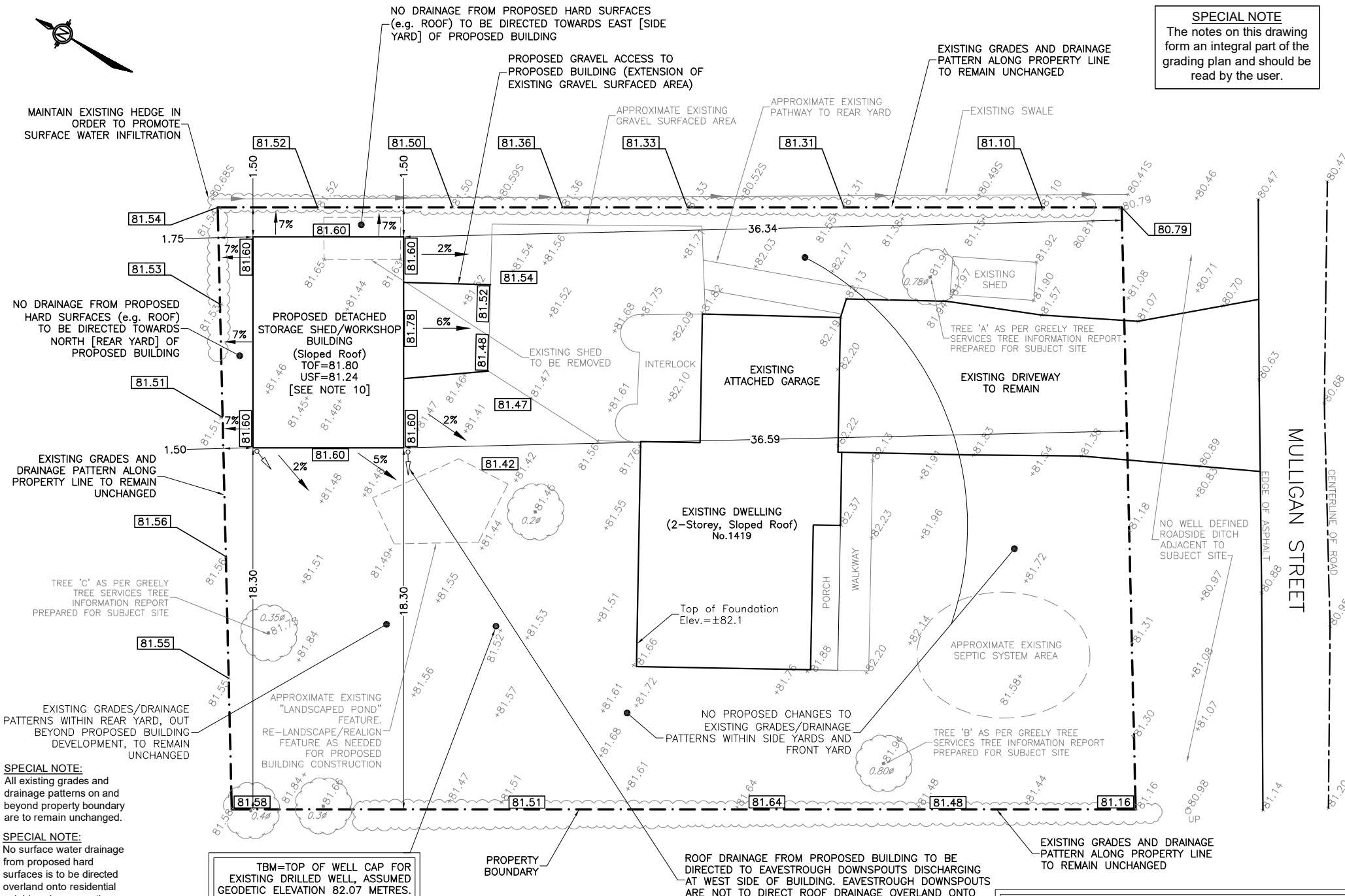
- 1- (5") concrete slab, specific concrete compressive strength, (SCCS) is 32 MPa, c/w 6x6 W7.4/7.4 (152x152 MW47.6/47.6) heavy duty wire mesh, min. clear cover is 2.0". The concrete matrix c/w 0.5% synthetic fiber. The slab sits on top of 2" rigid XPS insulation rated for R10, and min rated compressive strength is 275 KPa or higher, (Such as Formular 400 or equivalent). The rigid insulation sits on 6 mil Poly on top of 0.5" stone dust, on top of 6" type A gravel and 6" type B gravel compacted to 90 % proctor. The bottom of the slab perimeter, and gravel sit on undisturbed soil, no less than 12" deep or rock. If not specified by the architectural drawings, make slopes 1% of the slab towards the main door. Optional, after a week of casting apply a non slippery epoxy floor coat to protect concrete from staining.
- 2- Building structural frame, by others.
- 3- 12.5 mm ( 0.5") dia, L shaped 36" x 10"@ 12" O.C. peripheral rebar dowels.
- 4- 2-5/8" peripheral horizontal reinforcement rebar, goes all around the perimeter of the slab, overlapping 20" in a run. The slab peripheral top and bottom rebar consist of 2 rebar located at the bottom and 2 rebar located at the top, 3"-4" from bottom / top surface of the slab.
- 5- 6 mil Poly sits on top of 0.5" stone dust.
- 6- 6x6 W7.4/7.4 (152x152 MW47.6/47.6) heavy duty wire mesh, at the middle of the slab. Overlab 14" in a run. Minimum clear cover is 2.0".
- 7- Horizontal rigid thermal insulation, XPS, 2"thick rated for R10, and min rated compressive strength is 275 KPa or higher, (Such as Formular 400 or equivalent). The horizontal insulation to be installed underneath the whole slab, and footings and to extrude 54" all around the slab edges.
- 8- Metal roof, by others.
- 9- 12.0" clear stone compacted to 90% standard proctor, and 0.5" stone dust at the top, (0.5" stone dust on top of 6" type A gravel and 6" type B gravel).
- 10- 2" x 6" @16" O.C. standard stud wall c/w 2-2"x6" top plates and 2" x 6" bottom plate. The bottom plate is fixed to the diaphragm with 2- 0.25" x 2.25" @ 4" O.C. staggered SDS . The top plates is fixed to the corresponding steel frame elements, (Main frame and purlins), using metal screws. See wall table for details.
- 11- After 48 hours, saw cut 1/4" x 1" @ 11'-8"-12.5' continuous contraction joints (Spaced 11'-8" along the long side of the slab, while spaced 12.5' along the short side of the slab), filled with backer rod and silicon caulking.

**General Notes**

- Contractor is responsible to confirm all dimensions on structural drawings with actual site conditions before proceeding with work. Report inconsistencies or discrepancy to the designer or project engineer before proceeding. Do not scale drawings.
- The contractor is responsible for safety on the job site, and design, installation and supervision of all temporary bracing, shoring, scaffoldings, and supports, and safety on the job site for all occupants and employees.
- Any changes to the structural design must have the engineer's approval.
- Provide 4 days notice for visit inspection by Engineer, inspection fee may apply.
- All work and material are to be in compliance with all applicable codes, Regulations and by-laws.
- Additional drawings may be issued for clarification to assist the proper execution of work. These drawings will be considered as part of the original documents and additional fee may be required.
- Make smooth finish for slabs. Provide 1 % slopes towards the door or designated drain/s, if it is not determined by the architect.
- Design non-factored loads, LL= 100 psf, DL=20 psf, roof, SL=48.5 psf. Soil allowable capacity is assumed 1,200 psf, if different contact the engineer to make necessary changes to the slab design.
- All regular lumber is SPF grade No.1 or NO.2 unless specified otherwise.
- Structural design is based on dimensions and drawings supplied by the client on Feb. 26th, 24, and in accordance with amended version of the building steel frame drawings, (By others, Olympia Steel Building drawings dated 02/22/24) supplied by client on Feb. 25.
- Comply with the Ontario and Canada Health and Occupational Safety Act. The design is in accordance with OBC 2012.
- Aggregate to be free of organic impurities and any deleterious materials, and does not react with the Portland cement. Portland Cement Type I is used in general. If soil has sulphate in moderate quantity Type II must be used for the foundations and slabs on ground.
- All reinforced concrete of the foundations, footing and slabs are 32 MP with 7% air entrained. Allow 24 hrs for concrete to dry before stripping formwork. Allow three days for concrete to harden before foundation backfilling. Fresh concrete to be kept in 100% humidity condition for at least 28 days. Ambient temperature, concrete matrix, and the equipment used in placement of the concrete (which is in touch with the matrix) to be min + 10 C° (50 F°) or higher when casting concrete, which can be done in winter by sheltering the whole construction areas and using a gas heater. This temperature remains min 14 days for regular concrete and min 5 days for fast curing concrete, with SCCS of the cast concrete is no less than 21 MPa. before removing the shelter and heaters. Non chloride non corrosive accelerators might be used when casting the concrete at ambient temperature down to -2° C.
- Footings / bottom of the slab and compacted gravel at the bottom of the slab, to sit on undisturbed soil, or rock. All agricultural /organic soil to be removed.
- Structural steel rebar confirm to CSA -G30.12M grade 400. All non galvanised steel to be sand blasted, grated or steel brushed and scrubbed to remove all rust before using steel in fabrication of trusses and any other part of the buildings. All rebar to be sandblasted, grated or steel brushed and scrubbed to remove all rust before using the steel in the concrete.
- North direction is an architectural convention and may differ from the geographic north.
- Limitations  
 -The scope of our work and related responsibilities related to our work are only to design the slab on grade for the new garage building, and a loft, for the loads mentioned herein including the car lift. These drawings shall not be used to express or imply warranty as to the fitness of the property or any other parts thereof for a particular purpose, or / and design loads unless otherwise agreed in writing by the Engineer. The Client and other users of this drawings expressly deny any right to any claim against the Engineer whatsoever, including claims arising from personal injury caused by pollutants, contaminants or /and any other hazardous materials, including but not limited to asbestos, mold or /and other fungus.



Clint Millar-Langlois 1419 Mulligan St. Gloucester ON K1V 1H4, terrydaley1@gmail.com	<b>Project</b> <b>A slab on grade for a new garage</b>	<b>Ottawa Structural Engineering Services Inc</b> 200-100 Metcalfe St. Unit 353 Ottawa, ON K1P 5M1	Drawings and Structural Review, Sam Yakoub , Structural Engineer, Ph.D, PEng.	Title Notes	Drawing No <b>S5</b>
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**SPECIAL NOTE**  
The notes on this drawing form an integral part of the grading plan and should be read by the user.

- NOTES:**
- All dimensions and elevations are in metres, unless otherwise indicated. Do not scale drawing.
  - TBM = As shown/described on drawing, assumed Geodetic elevation 82.07 metres. Geodetic elevations shown on drawing are derived from the Can-Net VRS Real-Time GNSS network at the time of the fieldwork. Morey Associates Ltd. accepts no responsibility for any third party use of the above mentioned TBM.
  - Property boundary information and existing dwelling location shown on this drawing is from survey drawing titled "Part of Lot 15, Concession 1 Rideau Front, Township of Gloucester", dated August 5, 1983, prepared by Farley & Martin Ltd. OLS, provided to us by client. The proposed building location shown on this drawing is from a site plan sketch prepared by client and based on direction from client. The size of the proposed building shown on this drawing is from the Ottawa Structural Engineering Services Inc. foundation design drawings for project titled "A slab on grade for a new garage", dated March 4, 2024, provided to us by client by email dated March 7, 2024. The original topography/ground elevations, structure locations and existing site features shown on this plan are supplied for design and approval purposes only and assumed to be accurate. It shall be the responsibility of the contractor to verify the accuracy of all information obtained from plans for construction purposes.
  - This drawing is not a legal survey plan. This drawing is not a site control plan. This drawing is not a sewage system design. This drawing is not a landscape plan.
  - The intent of this grading plan drawing is to show surface water drainage directed away from the proposed building at the site.
  - All dimensions to be verified on site by contractor prior to construction.
  - Boundary information and proposed structures' locations/sizes and dimensions shown on this drawing have been provided to us or derived from information provided to us by others. Information provided to us by others is assumed to be accurate and verification of information provided to us by others is outside the scope of this drawing (see Note 21). Morey Associates Ltd. should be retained if dimensions verified on site by contractor differ from this drawing as this may require design changes.
  - Design and location of all utilities, such as but not limited to, hydro wires, telephone wires, cable wires, gas lines, underground services, etc., and easements are outside the scope of this grading plan drawing. Contractor is responsible for location and protection of all existing and proposed utilities and easements. Morey Associates Ltd. accepts no responsibility and no liability for damage to services, utilities, and structures due to construction operations. Morey Associates Ltd. accepts no responsibility and no liability for the groundwater quality and/or quantity from the existing well at the site.
  - Client is responsible for acquiring all necessary permits. This drawing is not for construction until all necessary permits have been acquired.
  - Top of foundation (TOF) and underside of slab on grade thickened edge (USF) elevations for the proposed building shown on drawing is based on the above mentioned foundation design drawings indicating a 0.56 metre thick thickened edge slab on grade foundation. Where less than 1.5 metres of earth cover above USF level is provided, rigid insulation in combination with earth cover may be required for footing subgrade frost protection purposes.
  - The underside of footing elevation and finished grade at the proposed building has been set based on limited information and may not have accounted for actual groundwater and/or soil/bedrock conditions at the proposed building location. It should be noted that groundwater levels are expected to fluctuate seasonally. Higher groundwater levels are expected during wet periods of the year such as the early spring. Contractor and/or owner is responsible for determining, prior to or at time of excavating, if the actual in-situ groundwater and/or soil/bedrock conditions at the proposed building location warrant changes to the USF elevation and/or finished grade at the proposed building. As such, if consideration is being given by the contractor and/or owner for changes to the USF elevation and/or finished grade at the proposed building, Morey Associates Ltd. should be retained as this may require changes to this drawing.
  - Finished grade to slope downwards and away from proposed building everywhere, whether or not indicated on this drawing.
  - Maximum allowable proposed landscape (overburden) slope on site is 3H:1V. Finished grade adjacent to proposed building to slope downwards and away from proposed building at all sides at a minimum of 2% and a maximum of 7% out beyond building a minimum 0.5 metres. Beyond 0.5 metres the finished grade slope downwards and away from proposed building may be increased up to 3H:1V.
  - The proposed grades have been set for subject site grading and drainage only and for the area at the subject site in close proximity to the proposed site development only. Surface water ponding may occur at the subject site out beyond the proposed site development area. All grading and drainage control beyond the proposed site development area and beyond the subject site property boundaries and within the Municipal roadway right-of-way is outside the scope of this grading plan and is the responsibility of the property owners and the Municipality, respectively.
  - No excess overland drainage, during and after construction should be directed onto the neighbours' properties and no alteration to existing grade and drainage pattern on or beyond property lines is to take place.
  - Contractor is to ensure eavestrough drainage outletting at downspouts is ultimately directed to a legal drainage outlet (ie: existing drainage ditch/roadside ditching system/drainage easement/historical drainage outlet/on-site infiltration/etc.), and that no eavestrough drainage outletting at downspouts is directed overland onto neighbouring properties. Contractor to ensure that proposed eavestroughs and downspouts are adequate to convey the proposed building roof drainage.
  - The soil subgrade conditions at the proposed building location should be verified as acceptable by qualified geotechnical personnel from an allowable soil bearing pressure point of view for the proposed building construction at the site. It is the responsibility of the contractor and/or owner to retain qualified geotechnical personnel to carry out the above prior to or at time of excavating.
  - This drawing has been prepared for the exclusive use of Clint Millar-Langlois for the purposes of obtaining a City of Ottawa building permit. This drawing has not been prepared for the purposes of contractors bidding on the construction of the proposed grading and drainage works. Contractors bidding on or undertaking the grading and drainage works should examine the information shown on this drawing, satisfy themselves as to the adequacy of the information for construction and how it affects their construction techniques, schedule, safety, equipment capabilities and costs.
  - By use of this drawing for construction of the project, the client and owner confirms that they have reviewed and approved the drawing and the contractor confirms that they have visited the site, familiarized themselves with the local conditions, verified field dimensions and correlated their observations with the requirements of the drawing.
  - This drawing provides a limited illustration of the work to be done to construct the proposed grading and drainage works. Morey Associates Ltd. is not responsible for the means, methods, techniques, sequences and/or procedures used to carry out the work, or the safety aspects of construction, and nothing on this drawing expressed or implied changes this condition. Contractor shall determine all conditions at the site and shall be responsible for knowing how they affect the work.
  - Morey Associates Ltd. accepts no responsibility for any deficiency, misstatement or inaccuracy shown on this drawing as a result of information provided to us by others. Morey Associates Ltd. accepts no responsibility for any damages and/or delays to construction due to any deficiency, misstatement or inaccuracy shown on this drawing as a result of information provided to us by others.
  - It is the responsibility of the contractor and/or owner and/or user of this drawing to obtain and follow the engineer's guidance with respect to any errors, omissions, inconsistencies, ambiguities or conflicts which are alleged regarding this drawing and with respect to actual in-situ conditions at the site as it relates to this drawing. The engineer waives any and all responsibility and liability for problems which arise from failure to follow this drawing, specifications and the design intent they convey, or for problems which arise from others' failure to obtain and/or follow the engineer's guidance with respect to any errors, omissions, inconsistencies, ambiguities or conflicts which are alleged and/or from others' failure to obtain and/or follow the engineer's guidance with respect to actual in-situ conditions at the site as it relates to this drawing.
  - Morey Associates Ltd. reserves the right to define and interpret any and all notes, values, lines, shapes and design intent on this drawing and those definitions and interpretations shall govern the use and intent of this drawing prior to, during, and after construction.
  - Any changes to this design/drawing must be verified and approved by Morey Associates Ltd. If any changes to this design/drawing are made without obtaining Morey Associates Ltd. written consent, the client and/or contractor shall assume full responsibility for the results of such changes and the client and contractor agrees to waive any claim against Morey Associates Ltd. and to release Morey Associates Ltd. from any liability arising directly or indirectly from such unauthorized changes. In addition, the client and contractor agrees, to the fullest extent permitted by law, to indemnify and hold harmless Morey Associates Ltd. from any damages, liabilities or cost, including reasonable attorney's fees and cost of defence, arising from such unauthorized changes.

**SPECIAL NOTE:**  
All existing grades and drainage patterns on and beyond property boundary are to remain unchanged.

**SPECIAL NOTE:**  
No surface water drainage from proposed hard surfaces is to be directed overland onto residential neighbouring properties.

TBM=TOP OF WELL CAP FOR EXISTING DRILLED WELL, ASSUMED GEODETIC ELEVATION 82.07 METRES.

ROOF DRAINAGE FROM PROPOSED BUILDING TO BE DIRECTED TO EAVESTROUGH DOWNSPOUTS DISCHARGING AT WEST SIDE OF BUILDING. EAVESTROUGH DOWNSPOUTS ARE NOT TO DIRECT ROOF DRAINAGE OVERLAND ONTO NEIGHBOURING PROPERTIES.

**LEGEND**

	EXISTING SPOT ELEVATION		PROPOSED DRAINAGE SLOPE
	EXISTING SWALE INVERT ELEVATION		PROPOSED EAVESTROUGH DOWNSPOUT(S) & DIRECTION
	PROPOSED FINISHED GROUND SURFACE ELEVATION		

**CITY OF OTTAWA TREE BY-LAW NOTE:**  
All trees on the Right-of-Way are to be maintained (unless a permit to remove the tree has been granted by the City of Ottawa) before and after construction and all trees within the property shall be protected as per the 'Municipal Trees and Natural Areas Protection By-Laws' and 'Urban Trees Conservation By-Law' as amended from time to time.

**CITY OF OTTAWA EROSION AND SEDIMENT CONTROL STANDARD NOTE:**  
The contractor shall implement best management practices, to provide for protection of the area drainage system and the receiving watercourse, during construction activities. The contractor acknowledges that failure to implement appropriate erosion and sediment control measures may be subject to penalties imposed by any applicable regulatory agency.

**EASEMENT NOTE:**  
Easement verification is the responsibility of the applicant. Easement information can be obtained from the Land Title and may be registered at the Ontario Land Registry Office. Express written consent is required from the easement holder to permit encroachment or modifications within the easement.

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DRAWING			
<b>GRADING PLAN</b>			
REV.	DRAWN BY	DATE	DESCRIPTION
LOCATION			
1419 MULLIGAN STREET RIVERSIDE SOUTH-FINDLAY CREEK WARD CITY OF OTTAWA ONTARIO			

PROJECT				
<b>PROPOSED DETACHED STORAGE SHED/WORKSHOP BUILDING</b>				
CLIENT				
CLINT MILLAR-LANGLAIS				
DATE	DRAWING No.	DRAWN BY	SCALE	FILE NO.
March 12, 2024	1 of 1	NNW	1:250	023249

**MOREY ASSOCIATES LTD.**  
CONSULTING ENGINEERS

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info@moreyassociates.com

### BUILDING SPECIFICATIONS

The manufacturer is not responsible for the concrete foundation design. The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The manufacturer will assume no responsibility for any loads not indicated.

This manufactured building is designed with the manufacturer's standard design practices which are based on pertinent procedures and recommendations of the following organizations and codes :

CSSBI – Standard for SBS 30M–06

–CSA S16–09, Limit States Design Of Steel Structures

–CSA Standard S136–07, Cold Formed

–Metal Building Manufacturers Association "Specification for the design fabrication and erection of the structural system" most current edition.

Material Specifications

ASTM A572 Grade 55 for Structural Plates galvanized G40/Z120

ASTM A653 Grade 55 for Cs and Zs Galvanized G40/Z120

ASTM A792 Grade 80 for roof and wall sheeting AZ 55/AZM 165 galvalume Steel Sheet

Erection: Building Erection shall be as per clause 29 of CSA S16–09. Erection of the building is not the responsibility of the building manufacturer.

### CONTRACTOR RESPONSIBILITIES

The contractor must secure all required approvals and permits from the appropriate agency as required.

Approval of the manufacturer's drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract drawings and specifications. (S16–09 (Appendix A)/CISC code of Standard Practice.)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (S16–09 (Appendix A)/CISC code of Standard Practice.)

Design considerations of any materials in the structure which are not furnished by the manufacturer, are the responsibility of the contractor and engineers other than the manufacturer's engineering, unless specifically indicated.

The contractor is responsible for all erection of steel and associated work in compliance with the manufacturer's "For Construction" drawings.

Temporary supports, such as guys, braces, flashwork or other elements required for the erection will be determined and furnished and installed by the erector. (S16–09 (Appendix A)/CISC code of Standard Practice.)

It is the contractors responsibility to apply or observe all pertinent safety rules and regulations, as per OSHA standards as applicable.

The Contractor is responsible for the verification of all shipments received. Any "external" damage or shortages must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim. NO EXCEPTIONS!!!

# Olympia Steel Buildings Canada

### DESIGN LOADING

This structure is designed utilizing the loads indicated and applied by the :

□NBC 2019 (NBC 15)

It is the contractor's responsibility to confirm that these loads comply with the requirements of the local building department.

Specific loads : (See structural calculations and foundation reactions.)

21.00 PSF Live Load

50.00 PSF Ground Snow Load, Rain Load (Sr) 8.3600 PSF

8.57 PSF Wind Load 1/50

2.200 PSF Dead Load (Metal Bldg. Weight – Purlins, Panels, Etc.)

3 PSF Collateral Load

II - Normal Use Category ( I = 1.00 )

### DRAWING INDEX

<u>CS-1</u>	Drawings Cover Sheet
<u>CS-2</u>	Fastener Schedule
<u>E1</u>	Anchor Bolt Plan
<u>E2</u>	Anchor Bolt Details & Reactions
<u>E3</u>	Rigid Frame Elevation
<u>E4</u>	Sidewall Framing
<u>E5</u>	Endwall Framing
<u>E6</u>	Roof Framing Plan
<u>E7</u>	Sidewall Sheeting
<u>E8</u>	Endwall Sheeting
<u>E9</u>	Detail Drawings
<u>E10</u>	Detail Drawings
<u>E11</u>	Trim Drawings

### SEISMIC DATA

1) Sa(0.2) = 0.44

2) Sa(0.5) = 0.237

3) Sa(1.0) = 0.12

4) Sa(2.0) = 0.056

5) PGA = 0.2810

6) Seismic Importance Factor: I= 1.00

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These Drawings are for :

- Construction  Approval \*  
 Permit  Anchor Bolts & Reactions  
 Other :

\* Approval orders must be released for fabrication within thirty (30) calendar days after the submittal drawings are issued or they will be subject to any current price increases. Special attention should be given in approving dimensions and/or details. Please verify requested dimensions by indicating 'OK'.

CAN/CSA A660 Certificate # RNMAN0

T&Z Consulting Services, LLC  
Ontario CofA No. 100521725

DSN: MQZ

DWN: SF

REV:

DET: SSN

CHK: AS

REVISIONS

### Engineering Seal

This certification covers parts manufactured and delivered by the manufacturer only, and excludes parts such as doors, windows, foundation design and erection of the building. The buyer is responsible for ensuring all specified loads are in compliance with regulatory authorities.



T & Z Consulting Services, LLC  
1428 N Shevlin Court  
Sewickley, PA 15143

NO. DATE

SCALE:  
NOT TO SCALE

DATE:  
2/27/24

JOB NO:  
011353

SHT. NO:  
CS-1

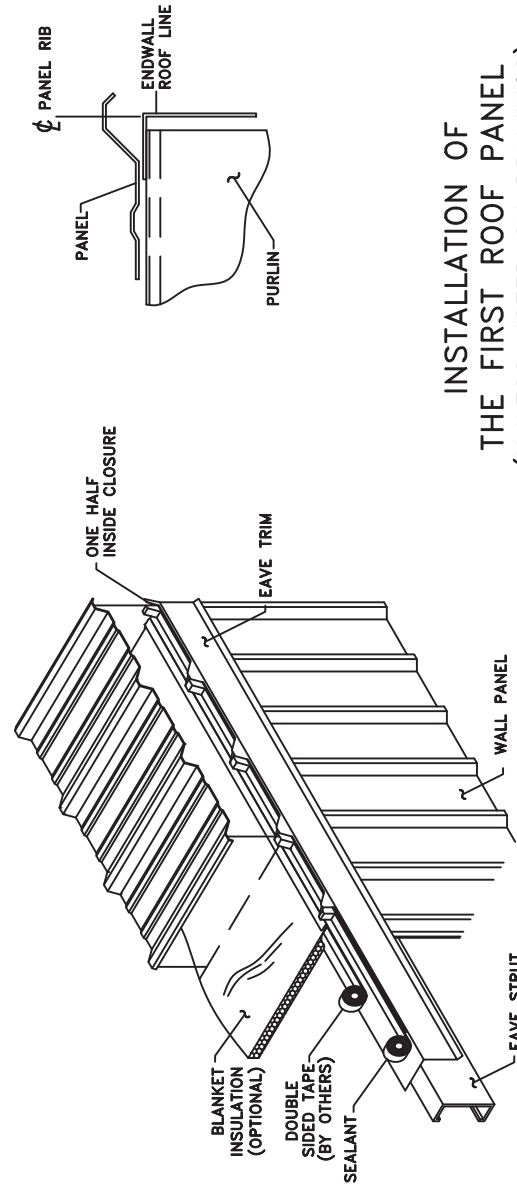
### DRAWINGS COVER SHEET

CUSTOMER :

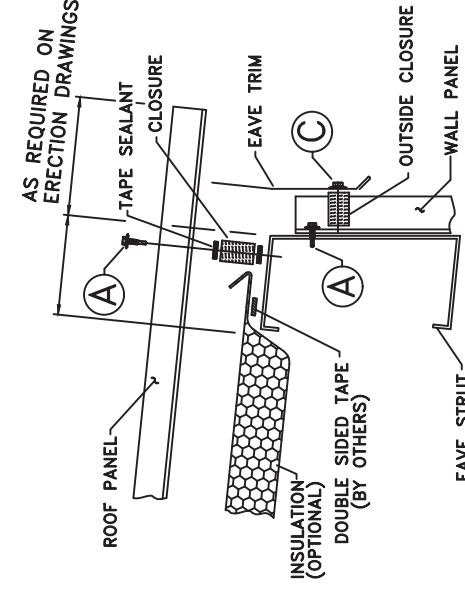
Clint Millar–Langlois  
1419 Mulligan St, Gloucester  
Ontario CANADA K1V 1H4

Olympia Steel Buildings Canada  
60 Renfrew Drive Suite 210  
Markham ON L3R 0E1





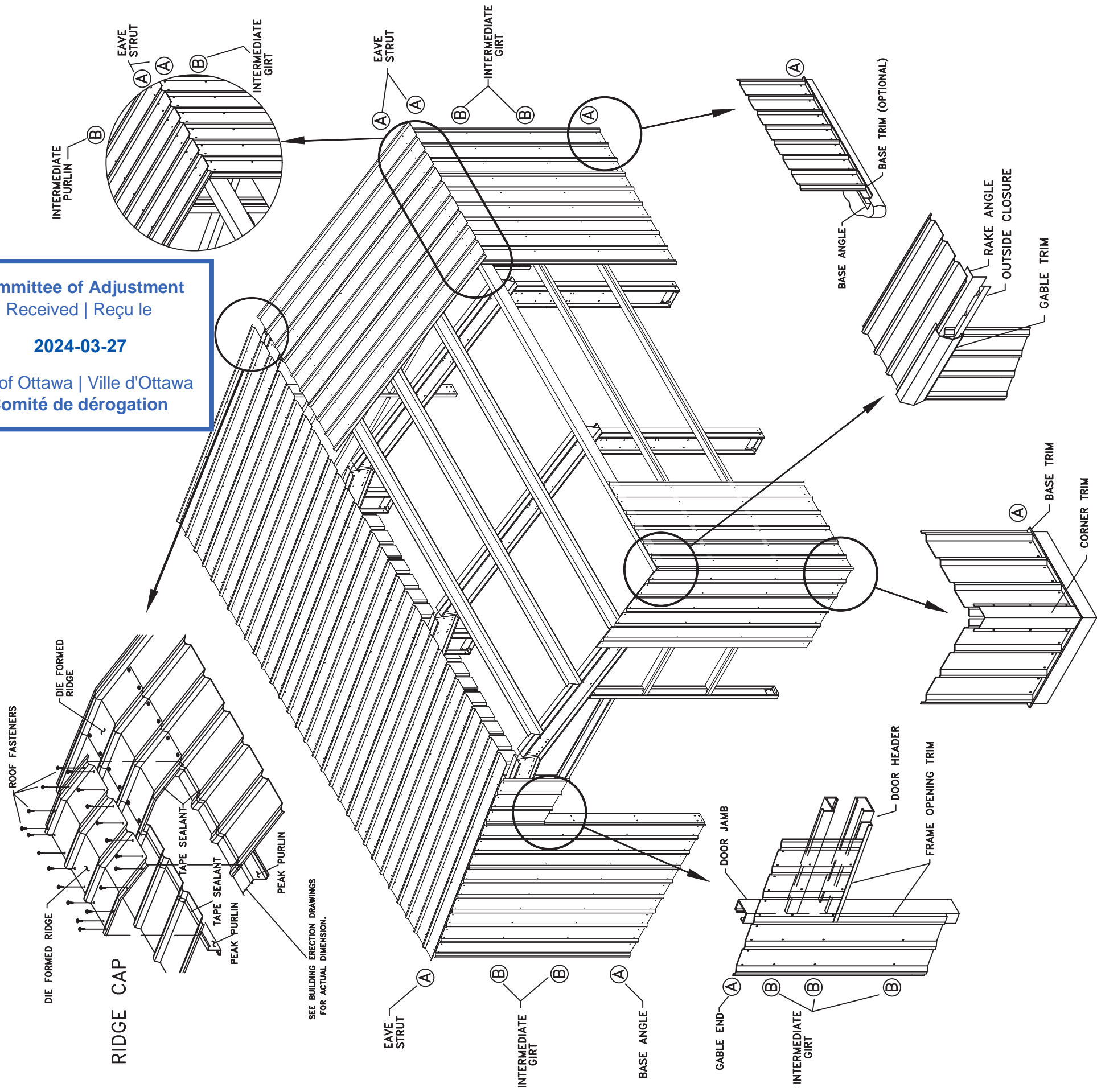
INSTALLATION OF THE FIRST ROOF PANEL (UNLESS NOTED ON DRAWINGS)



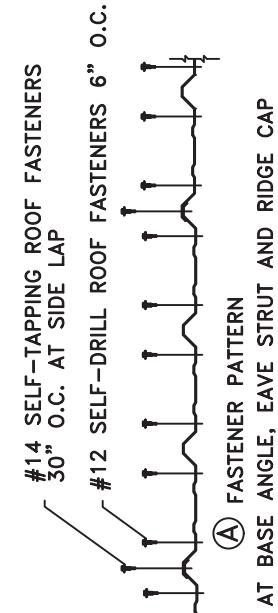
SECTION AT EAVE

# FASTENER SCHEDULE

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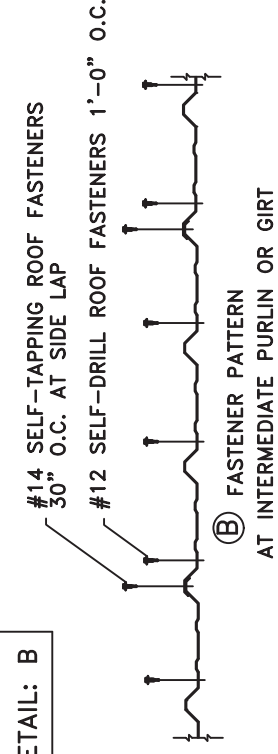


DETAIL: A



#14 SELF-TAPPING ROOF FASTENERS 30" O.C. AT SIDE LAP  
 #12 SELF-DRILL ROOF FASTENERS 6" O.C. AT BASE ANGLE, EAVE STRUT AND RIDGE CAP  
 FASTENER PATTERN AT BASE ANGLE, EAVE STRUT AND RIDGE CAP

DETAIL: B



#14 SELF-TAPPING ROOF FASTENERS 30" O.C. AT SIDE LAP  
 #12 SELF-DRILL ROOF FASTENERS 1'-0" O.C. AT INTERMEDIATE PURLIN OR GIRT  
 FASTENER PATTERN AT INTERMEDIATE PURLIN OR GIRT

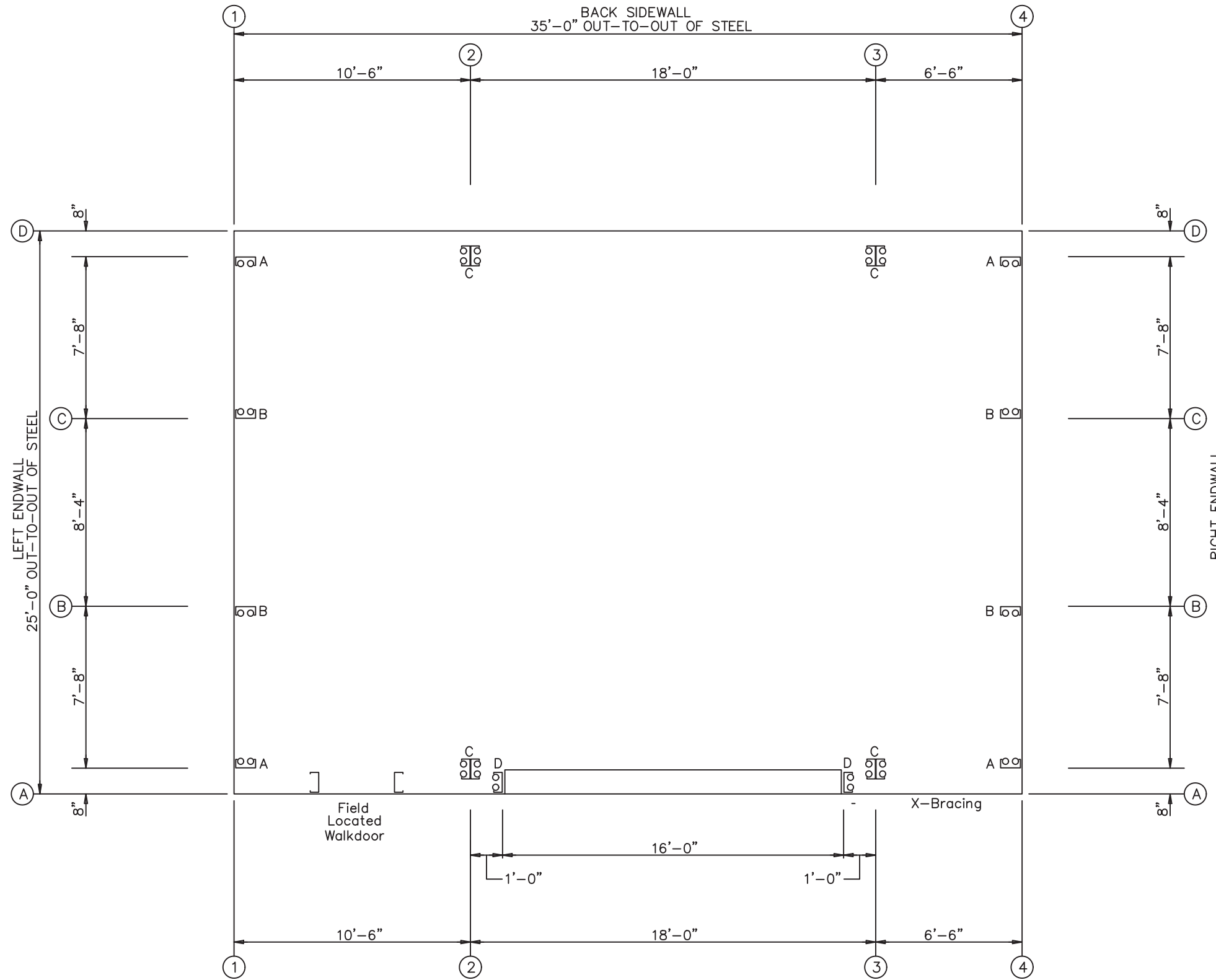
DETAIL: C

© STITCH FASTENER #14x7/8" 12" ON CENTER

DATE: 2/27/24

DWG NO:

CS-2



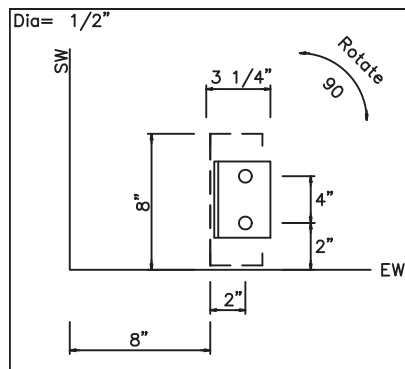
ANCHOR BOLT PLAN  
NOTE: All Base Plates @ 100'-0" (U.N.)

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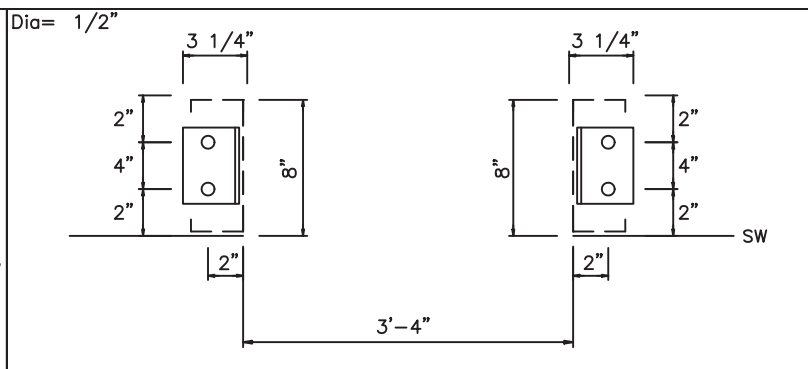


03/01/2024

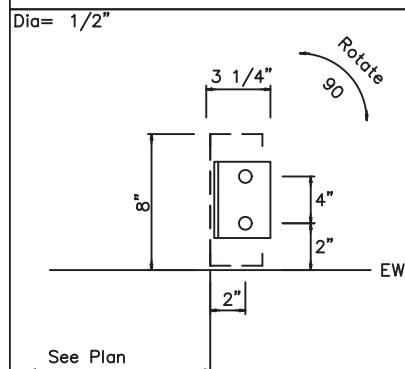
Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
ANCHOR BOLT PLAN			Sht E1 of 11



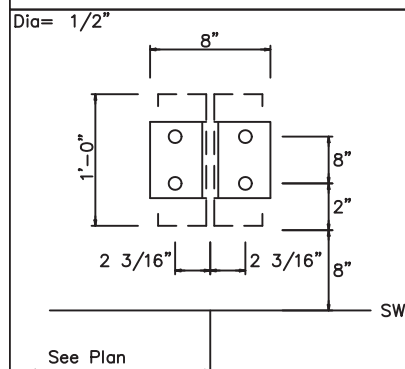
DETAIL A



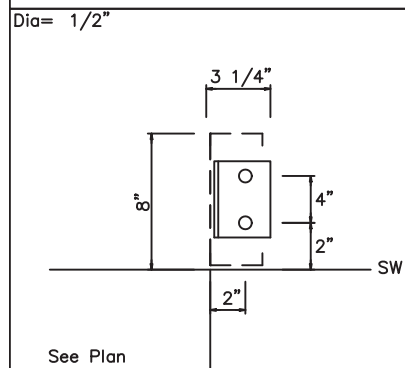
DETAIL OF FIELD LOCATED WALKDOOR



DETAIL B



DETAIL C



DETAIL D

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**NOTES FOR REACTIONS**

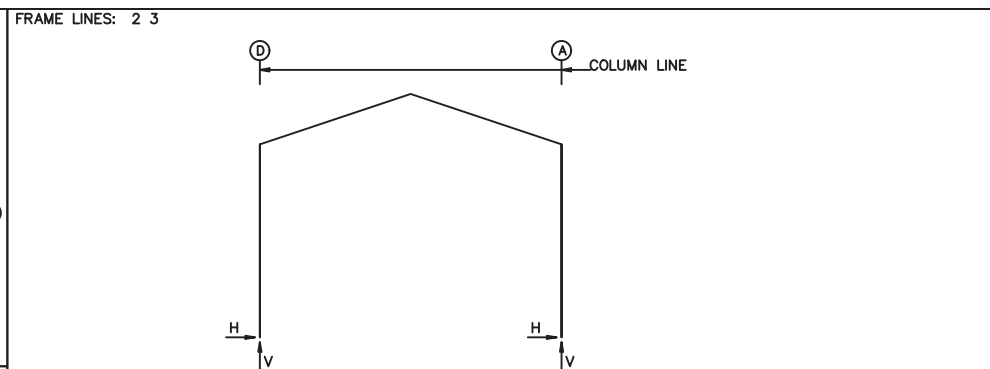
Building reactions are based on the following building data:

Width (ft)	=	25
Length (ft)	=	35
Eave Height (ft)	=	16 / 16
Roof Slope	=	4.0:12 / 4.0:12
Dead Load (psf)	=	2.200
Collateral Load (psf)	=	3
Roof Live Load (psf)	=	21.00
Roof Snow Load (psf)	=	48.49
Wind Load 1:50(psf)	=	8.57
Wind Code	=	ONBC 2019 (NBC 15)
Wind Category	=	2
Exposure	=	0
Importance - Wind	=	1.00
Importance - Seismic	=	1.00
Seismic Data	=	Sa(0.2) = 0.44
	=	Sa(0.5) = 0.237
	=	Sa(1.0) = 0.12
	=	Sa(2.0) = 0.056

- GENERAL NOTES**
- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
  - THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
  - THE SPECIFIED ANCHOR BOLT DIAMETER ASSUMES ASTM A307. ANCHOR BOLT MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER.
  - ANCHOR BOLTS TO BE SUPPLIED BY OTHERS. ANCHOR BOLT EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION ENGINEER.
  - ANCHOR BOLT PROJECTION ABOVE CONCRETE FINISHED SURFACE TO BE 3" UNLESS OTHERWISE NOTED BY FOUNDATION DESIGNER.
  - ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN ELEVATION AND LOCATION.
  - THE ANCHOR BOLT LOCATIONS PROVIDED BY THE METAL BUILDING MANUFACTURER MAY NOT SATISFY ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF FOUNDATION DESIGN. IT IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER TO MAKE SURE THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED IN THE FOUNDATION DESIGN.
  - MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.
  - THIS DRAWING IS NOT TO SCALE.

**BUILDING BRACING REACTIONS**

Loc	Line	± Reactions(k)				Panel Shear (lb/ft)	
		Wind	Seismic	Wind	Seismic	Wind	Seis
L_FW	1					30	29
F_SW	A	3.4	1.3	2.7	1.6	19	20
R_EW	4					36	60
B_SW	D						



**RIGID FRAME: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate Width	Base Plate Length	Base Plate Thick	Grout (in)
2*	D	4	0.500	8.000	9.000	0.250	0.0
2*	A	4	0.500	8.000	9.000	0.250	0.0

2\* Frame lines: 2 3

**RIGID FRAME: BASIC COLUMN REACTIONS (k)**

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	D	0.1	0.8	0.1	0.6	0.6	3.7	1.3	8.6	-1.3	-3.4	1.5	-1.2
2*	A	-0.1	0.8	-0.1	0.6	-0.6	3.7	-1.3	8.6	-1.5	-1.2	1.3	-3.4

**ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)**

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz	Wind Suct Horiz	Wind Long1 Vert	Wind Long2 Vert
1	D	0.1	0.1	0.3	0.8	-0.3	-0.2	0.2	0.3	-0.4	0.3	-0.7	-0.7
1	C	0.2	0.2	1.1	2.5	-1.0	-0.8	-0.8	-0.5	-0.9	0.6	-0.7	-0.7
1	B	0.2	0.2	1.1	2.5	-0.8	-1.0	-0.5	-0.8	-0.9	0.6	-0.7	-0.7
1	A	0.1	0.1	0.3	0.8	-0.2	-0.3	0.3	0.2	-0.4	0.3	-0.7	-0.7

**ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate Width	Base Plate Length	Base Plate Thick	Grout (in)
1	D	2	0.500	3.250	6.000	0.250	0.0
1	C	2	0.500	3.250	6.000	0.250	0.0
1	B	2	0.500	3.250	6.000	0.250	0.0
1	A	2	0.500	3.250	6.000	0.250	0.0
4	A	2	0.500	3.250	6.000	0.250	0.0
4	B	2	0.500	3.250	6.000	0.250	0.0
4	C	2	0.500	3.250	6.000	0.250	0.0
4	D	2	0.500	3.250	6.000	0.250	0.0



03/01/2024

**LOAD CASES**

WIND\_L1 = WIND LOAD FROM LEFT CASE 1  
 WIND\_R1 = WIND LOAD FROM RIGHT CASE 1  
 WIND\_LN1 = LONGITUDINAL WIND CASE 1

SEISMIC\_L = SEISMIC LOAD FROM LEFT  
 SEISMIC\_R = SEISMIC LOAD FROM RIGHT  
 SEISMICLN = LONGITUDINAL SEISMIC LOAD

F1UNB\_SL\_L = FRAME 1 UNBALANCED SNOW LEFT SIDE  
 F1UNB\_SL\_R = FRAME 1 UNBALANCED SNOW RIGHT SIDE  
 F1CRANE 1 = FRAME 1 CRANE LOAD IN POSITION 1

DRIFT = SNOW DRIFT LOAD  
 SLIDE = SLIDE SNOW LOAD

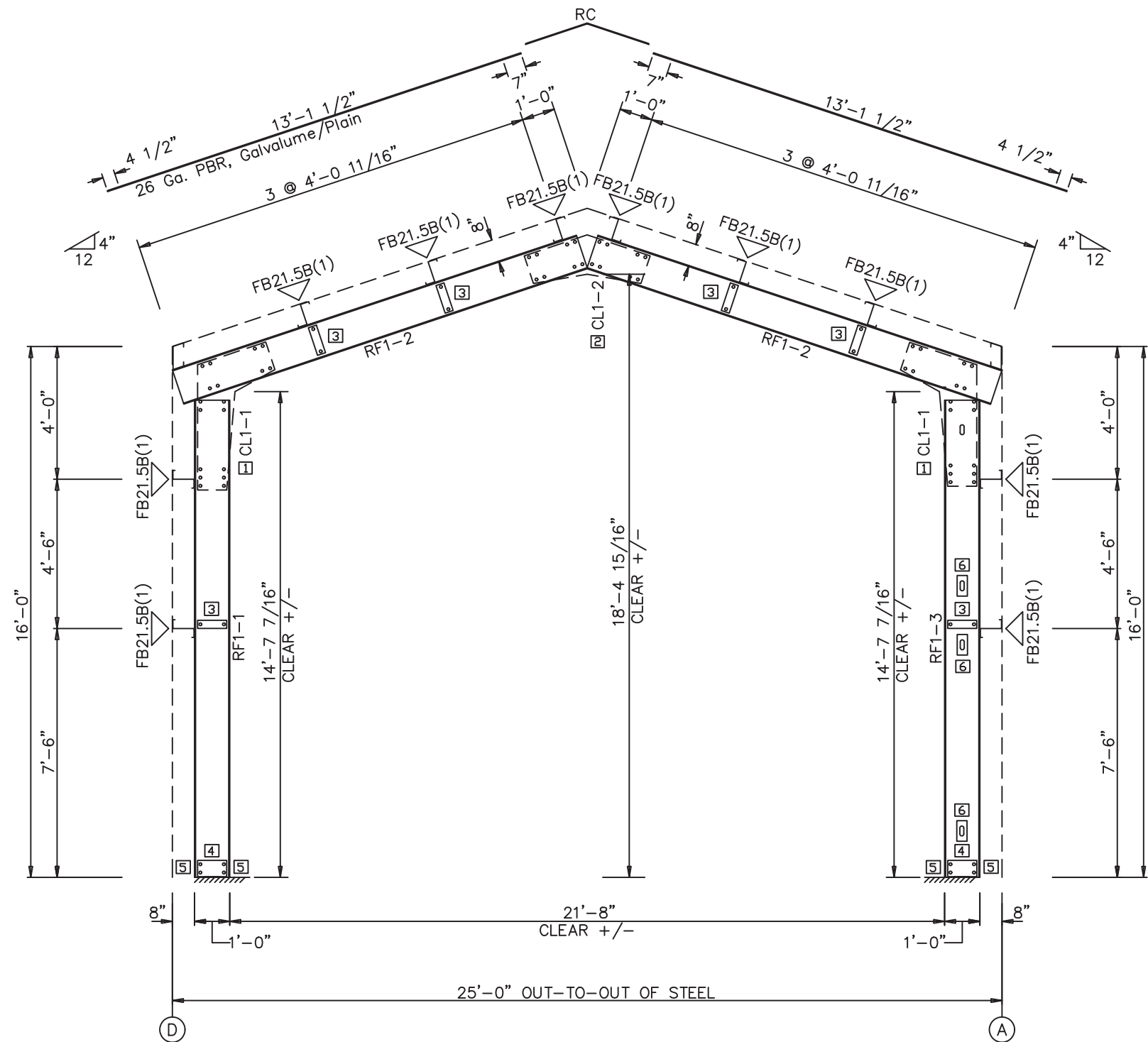
Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
<b>ANCHOR BOLT DETAILS &amp; REACTIONS</b>			Sht E2 of 11

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
CL1-1	9	9	0	A325	0.500	2.00
CL1-2	8	8	0	A325	0.500	2.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	12x55D12	14'-5 9/16"
RF1-2	12x55D10	12'-10 1/8"
RF1-3	12x55D12	14'-5 9/16"

✓ FLANGE BRACES: Both Sides(U.N.)  
 FB21.5B(1): 21.5=length(in), (1)=one side  
 B - L2X2X10G

CONNECTION PLATES	
ID	Mark/Part
1	CL1-1
2	CL1-2
3	CLC103
4	CLC102
5	CLC001
6	CLC110



RIGID FRAME ELEVATION: FRAME LINE 2 3  
 \* CLC110 USE ONLY GRID LINE 3

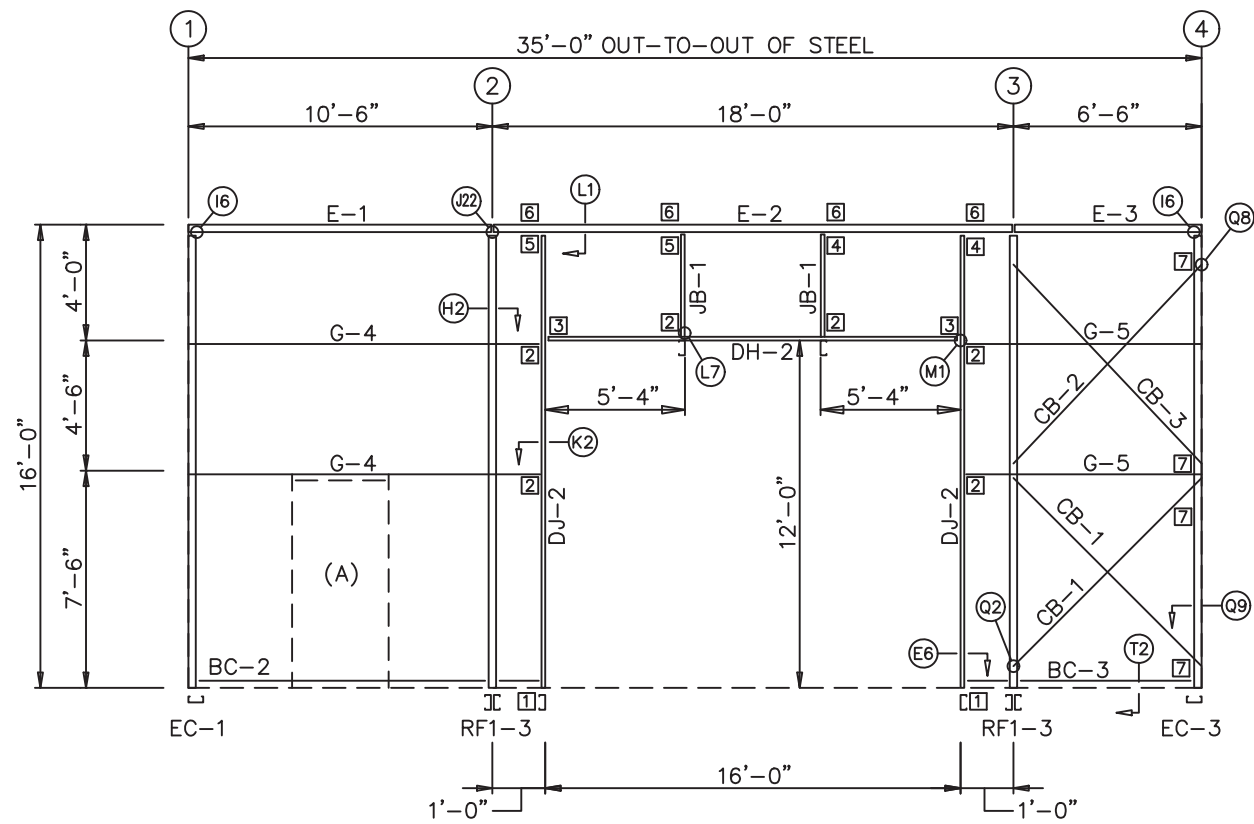
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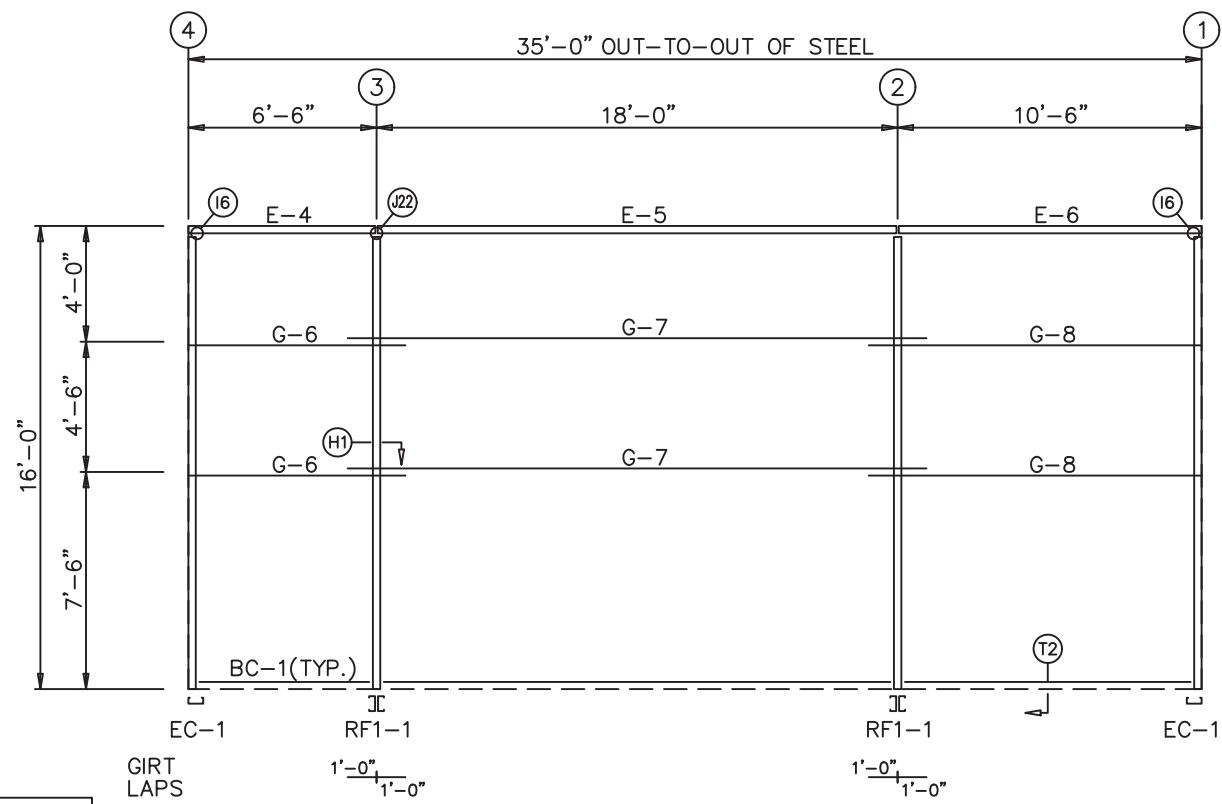
03/01/2024

**GENERAL NOTES:**  
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

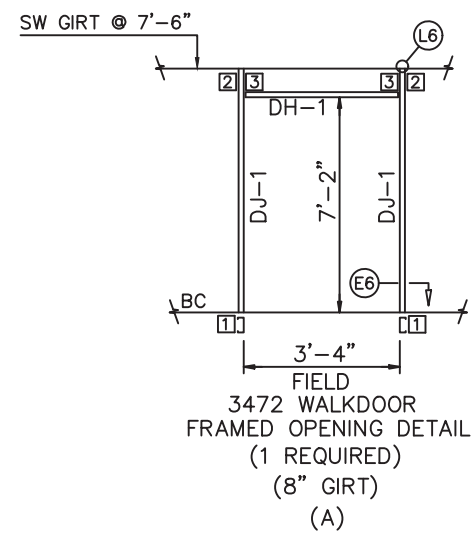
Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
RIGID FRAME ELEVATION			Sht E3 of 11



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D



MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
DJ-1	8x275C16	7'-1 1/2"
DJ-2	8x275C16	15'-3 3/8"
DH-1	8x275C16	3'-3 1/2"
DH-2	8x275C16	15'-11 1/2"
E-1	8x275E16	10'-5 1/4"
E-2	8x275E14	17'-11 1/2"
E-3	8x275E16	6'-5 1/4"
E-4	8x275E16	6'-5 1/4"
E-5	8x275E14	17'-11 1/2"
E-6	8x275E16	10'-5 1/4"
G-4	8x25Z16	11'-1"
G-5	8x25Z16	7'-1"
G-6	8x25Z16	7'-5 1/2"
G-7	8x25Z16	20'-0"
G-8	8x25Z16	11'-5 1/2"
CB-1	0.25_CBL	6'-10"
CB-2	0.25_CBL	6'-7"
CB-3	0.25_CBL	6'-6"
JB-1	8x257C16	2'-10 7/8"
BC-1	8x275C16	17'-6"
BC-2	8x275C16	11'-6"
BC-3	8x275C16	7'-6"

CONNECTION PLATES FRAME LINE A & D	
ID	MARK/PART
1	CLC008
2	CLC062
3	CLC025
4	CLC212
5	CLC213
6	SA
7	CLC253

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**GENERAL NOTES:**  
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Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
<b>SIDEWALL FRAMING</b>			Sht E4 of 11

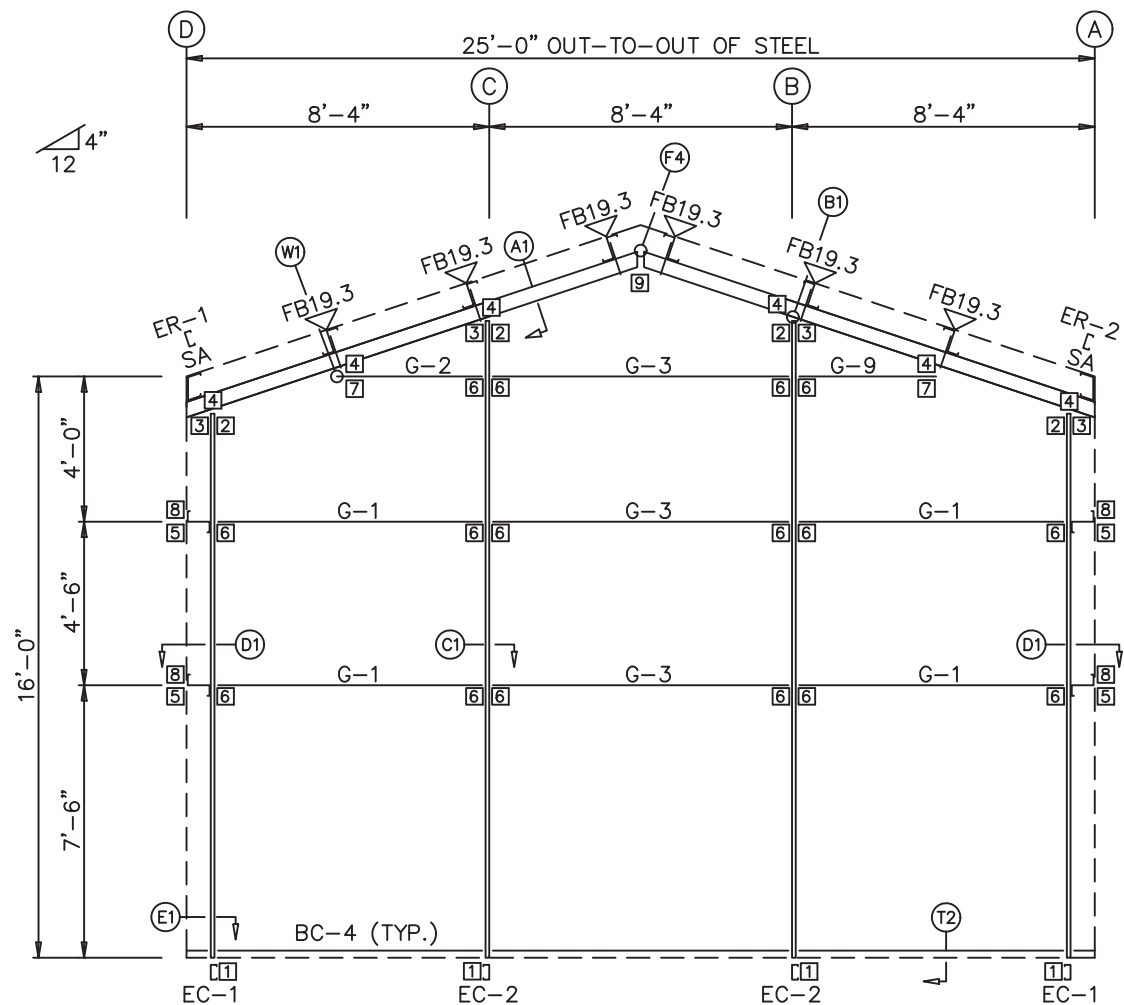
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BOLT TABLE FRAME LINE 1 & 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325T	1/2"	2"
Columns/Raf	8	A325T	1/2"	2"

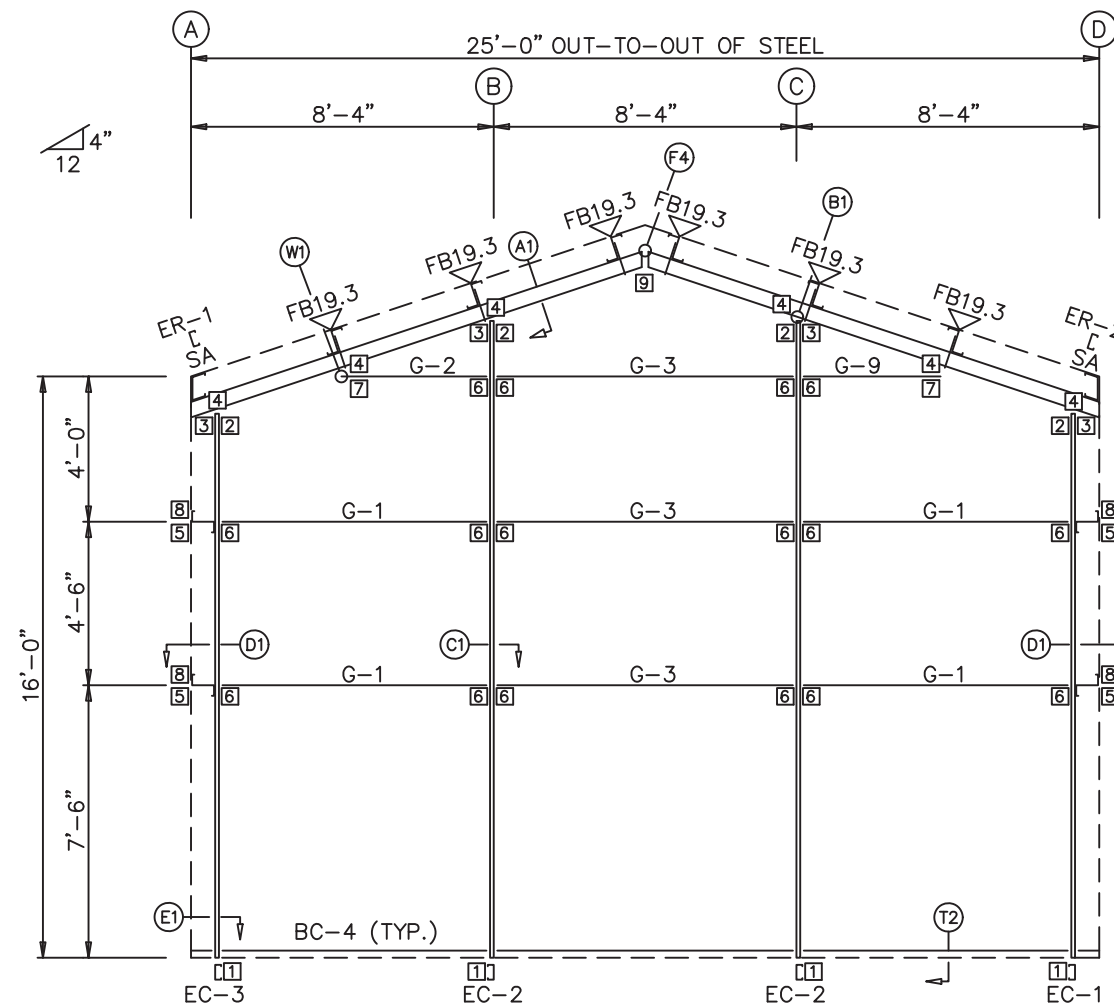
MEMBER TABLE FRAME LINE 1 & 4		
MARK	PART	LENGTH
EC-1	8x275C16	14'-8 13/16"
EC-2	8x275C15	17'-3 7/16"
EC-3	8x275C16	14'-8 13/16"
ER-1	8x275C16	12'-11 5/16"
ER-2	8x275C16	12'-11 5/16"
G-1	4x25Z18	6'-11"
G-2	4x25Z18	2'-8 7/8"
G-3	4x25Z18	8'-3"
G-9	4x25Z18	2'-8 7/8"
BC-4	4x25C18	12'-6"

FLANGE BRACE TABLE FRAME LINE 1 & 4			
VID	QUAN	MARK	LENGTH
1	12	FB19.3	1'-7 1/4"

CONNECTION PLATES FRAME LINE 1 & 4	
ID	MARK/PART
1	CLC008
2	CLC081
3	CLC083
4	CLC089
5	CLC054
6	CLC062
7	SCL-2
8	CLC053
9	CLC125



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 4



03/01/2024

**GENERAL NOTES:**

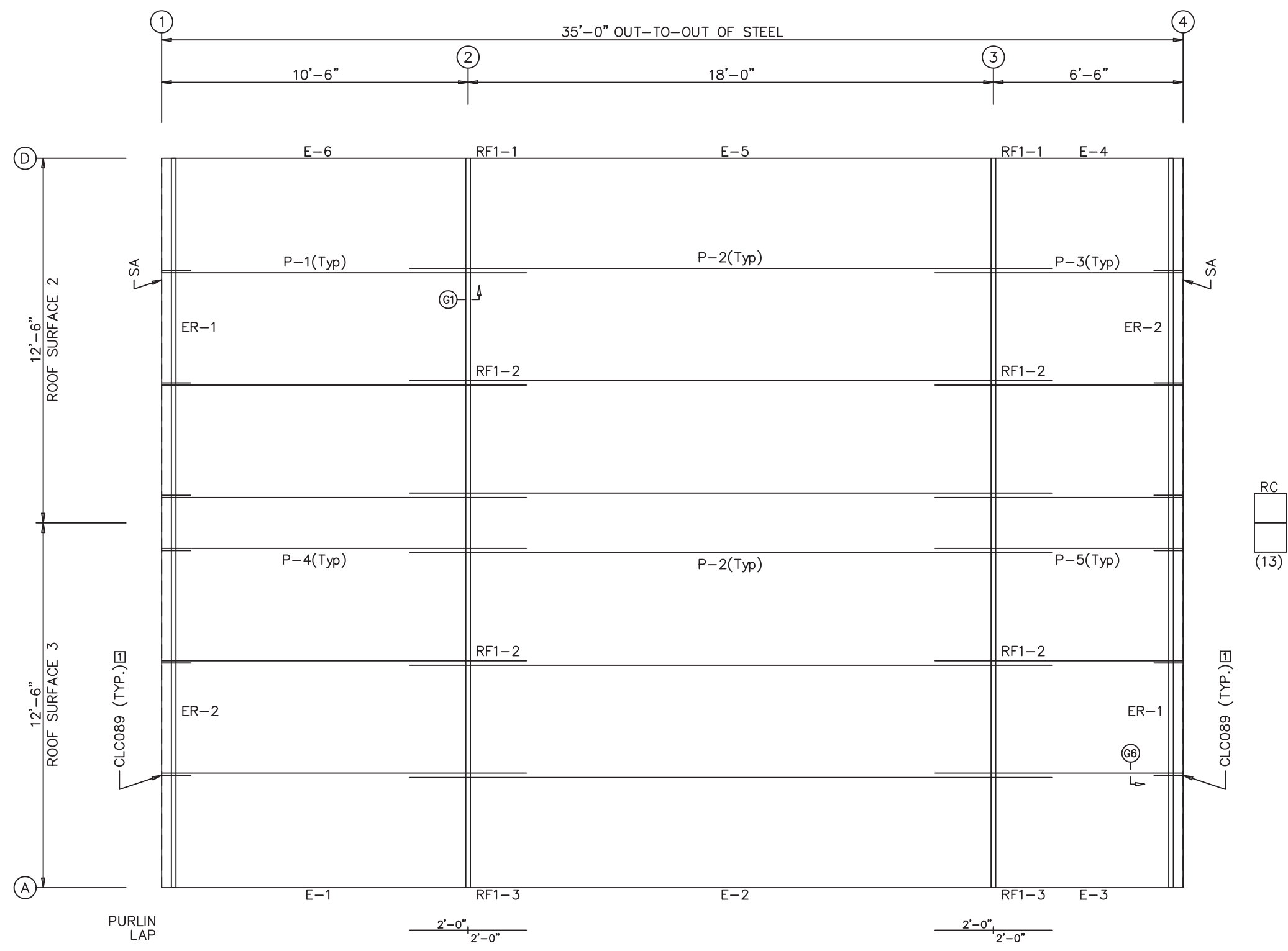
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
<b>ENDWALL FRAMING</b>			Sht E5 of 11

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z16	12'-5 1/2"
P-2	8x25Z16	22'-0"
P-3	8x25Z16	8'-5 1/2"
P-4	8x25Z16	12'-5 1/2"
P-5	8x25Z16	8'-5 1/2"
E-1	8x275E16	10'-5 1/4"
E-2	8x275E14	17'-11 1/2"
E-3	8x275E16	6'-5 1/4"
E-4	8x275E16	6'-5 1/4"
E-5	8x275E14	17'-11 1/2"
E-6	8x275E16	10'-5 1/4"

CONNECTION PLATES	
ROOF PLAN	
ID	MARK/PART
1	CLC089

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13'-1 1/2" (12)

13'-1 1/2" (12)

RC  
(13)

ROOF FRAMING PLAN

ROOF SHEETING  
 PANELS: 26 Ga. PBR Galvalume/Plain

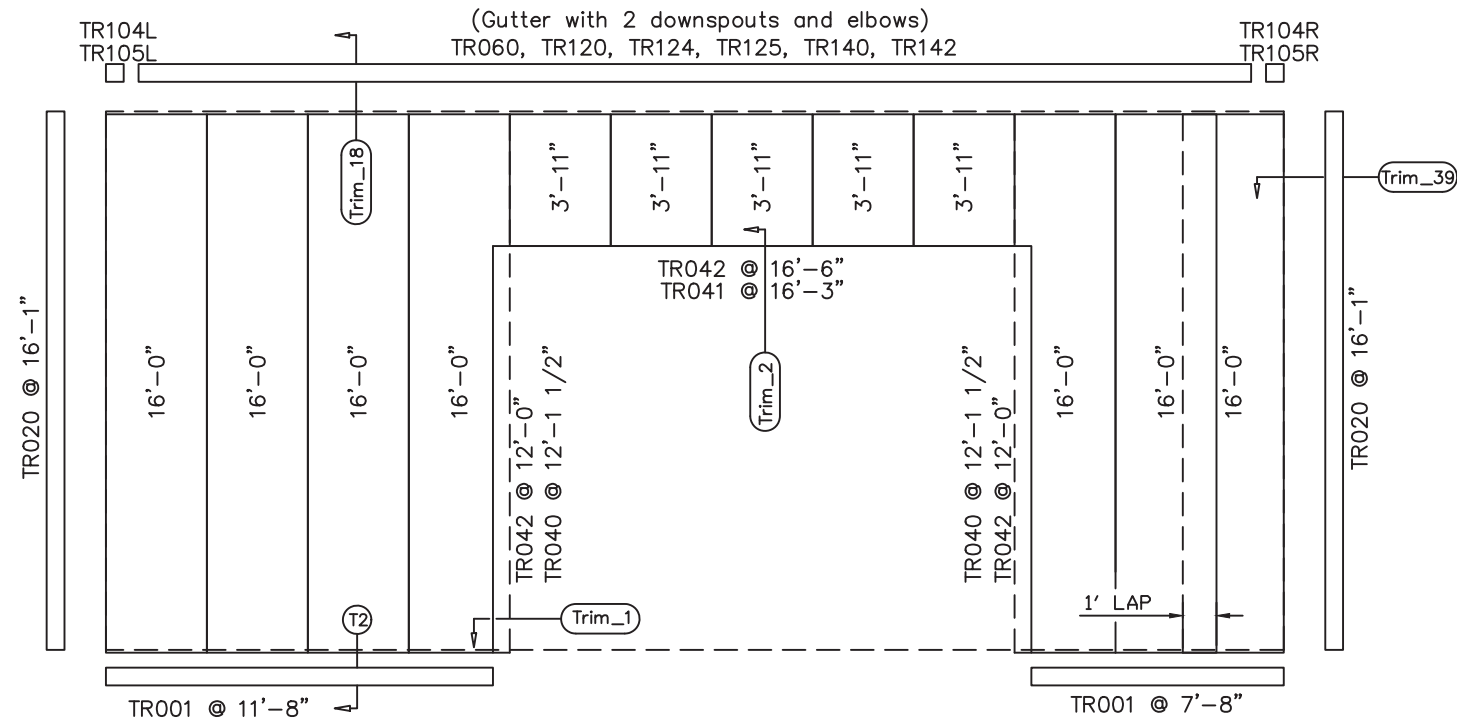


03/01/2024

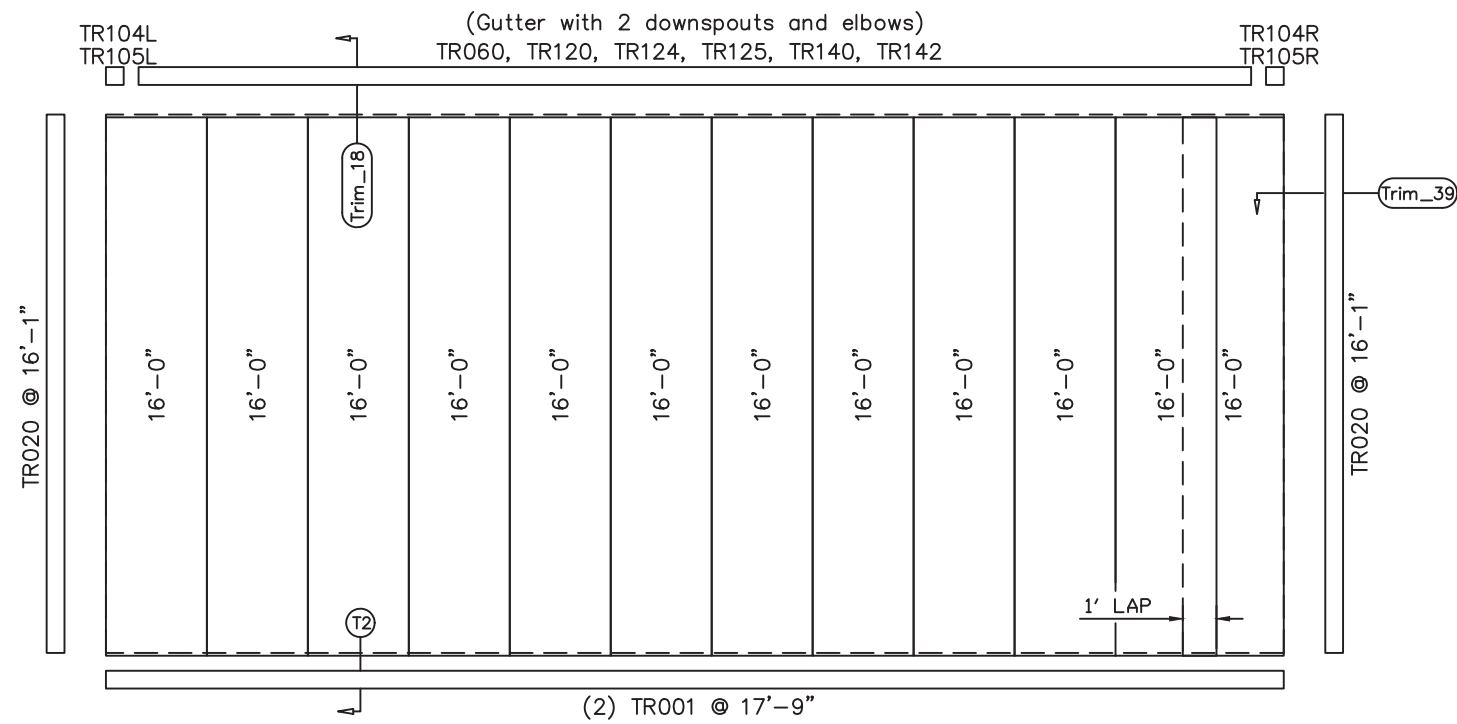
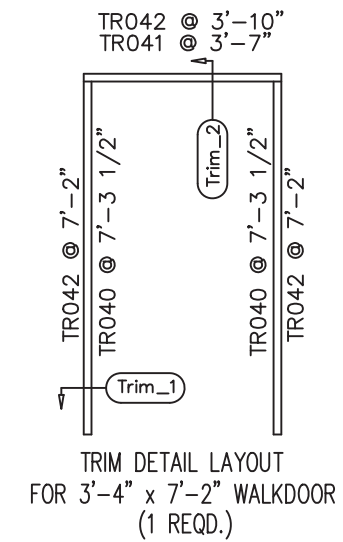
**GENERAL NOTES:**  
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID: 011353
Checker: AS	Date: 2/27/24		
ROOF FRAMING PLAN			Sht E6 of 11

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**SIDEWALL SHEETING & TRIM: FRAME LINE A**  
 PANELS: 26 Ga. PBR – Slate Gray



**SIDEWALL SHEETING & TRIM: FRAME LINE D**  
 PANELS: 26 Ga. PBR – Slate Gray



03/01/2024

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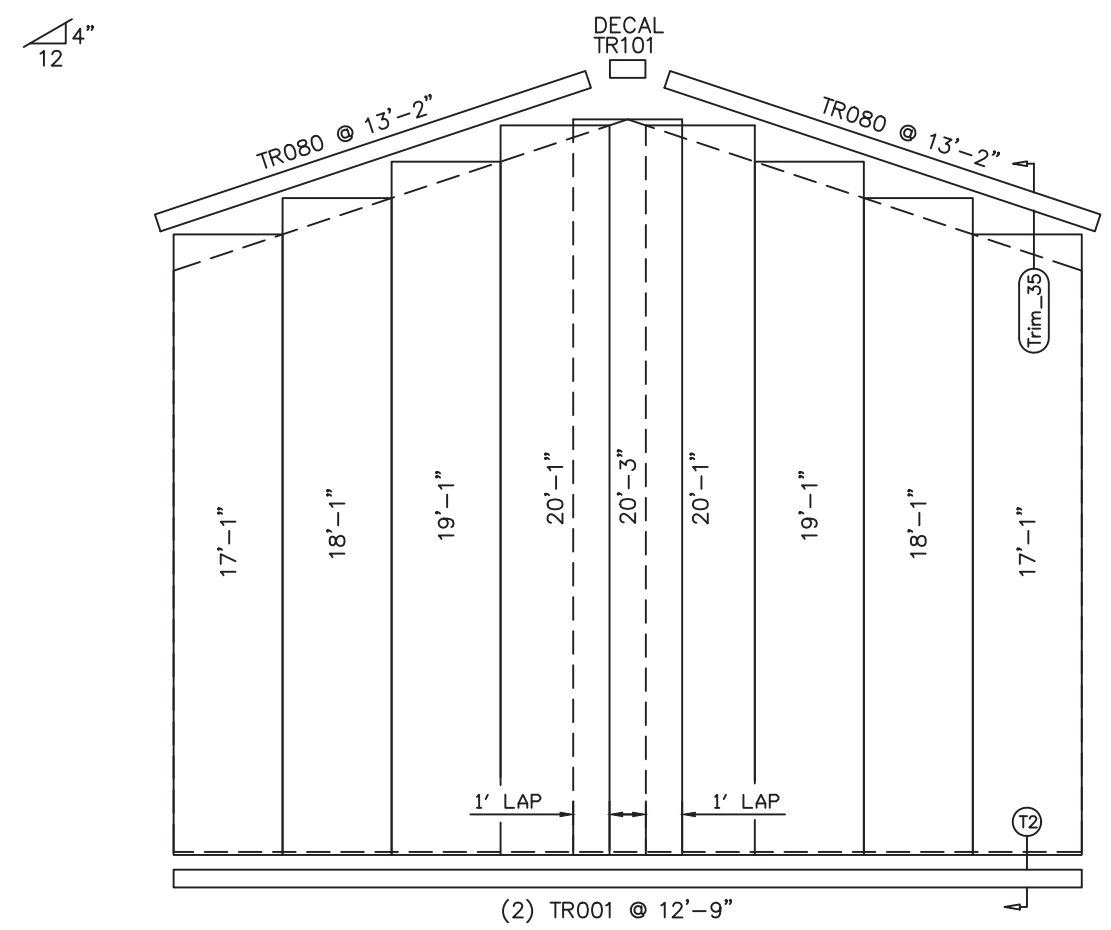
Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
SIDEWALL SHEETING			Sht E7 of 11



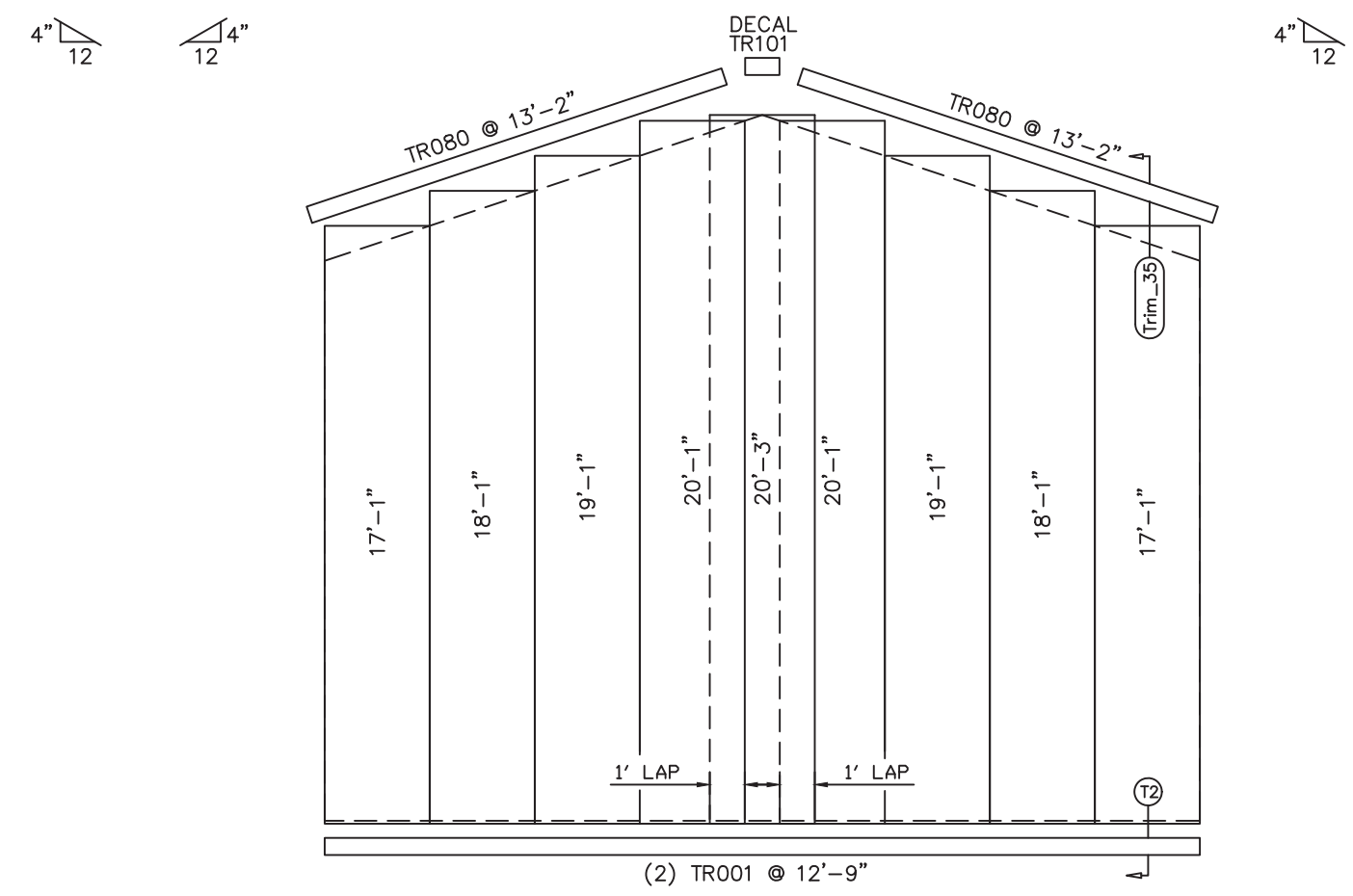
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Comité de dérogation



ENDWALL SHEETING & TRIM: FRAME LINE 1  
PANELS: 26 Ga. PBR - Slate Gray



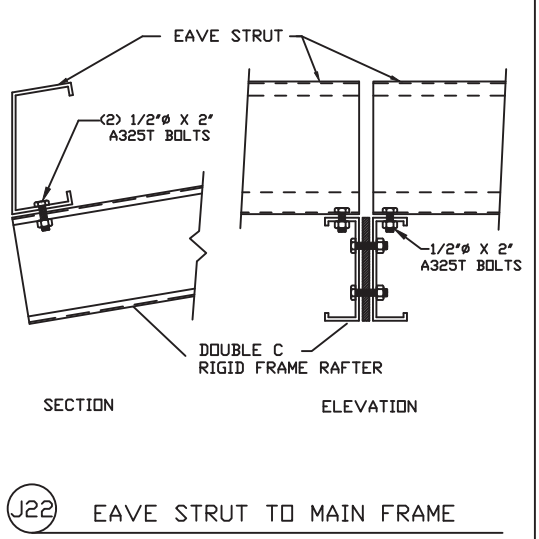
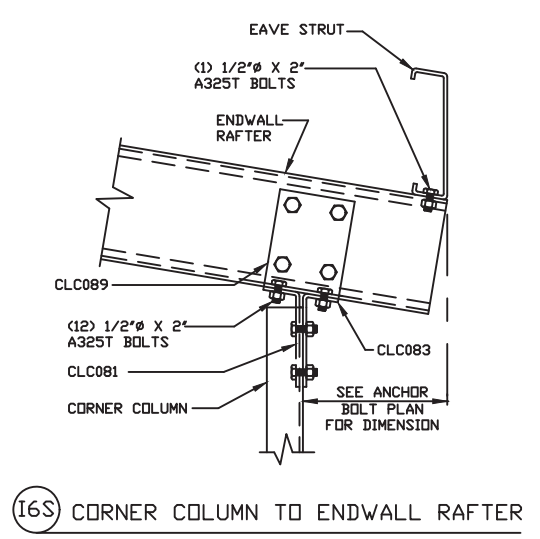
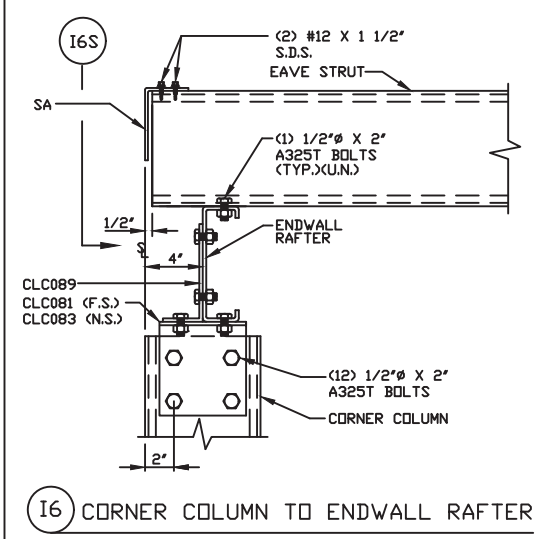
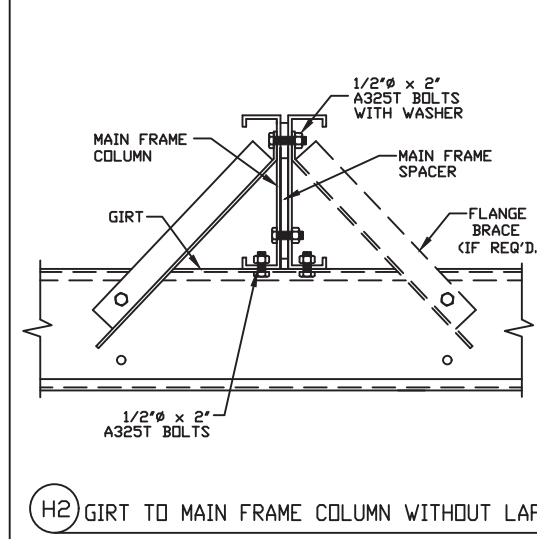
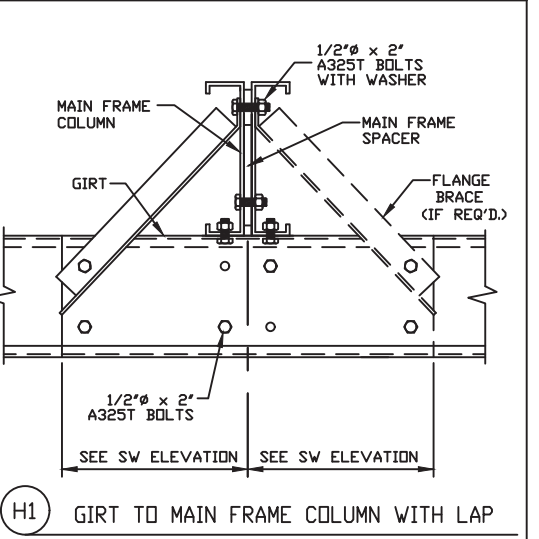
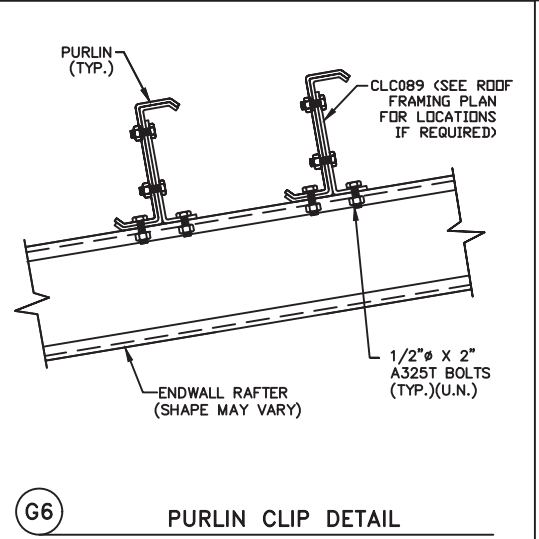
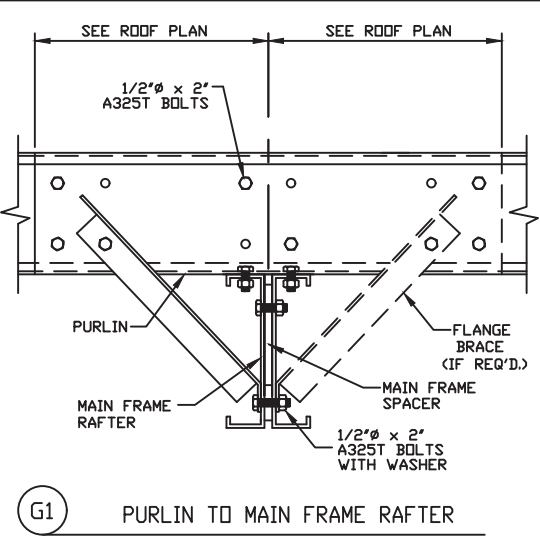
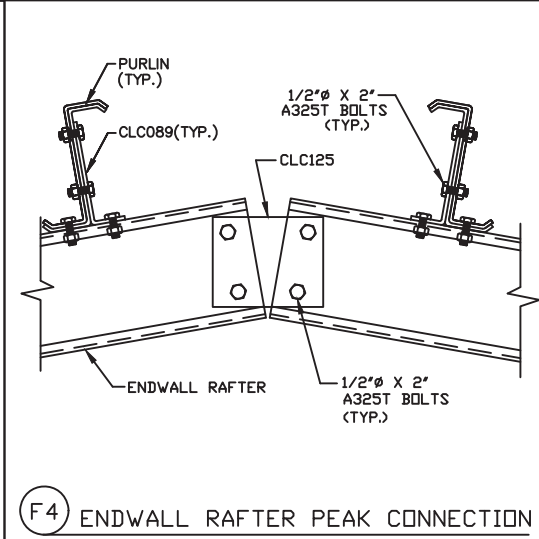
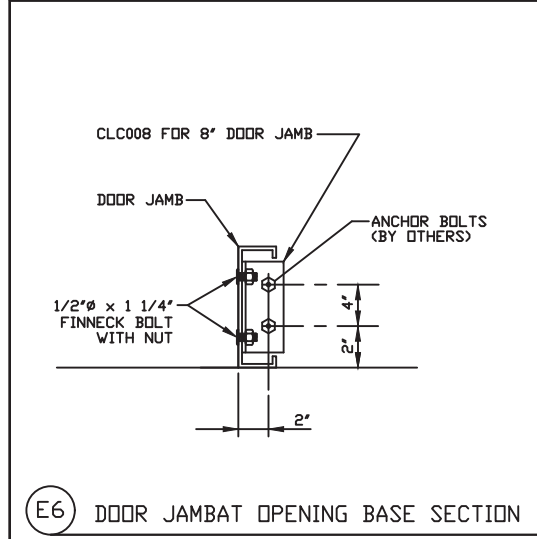
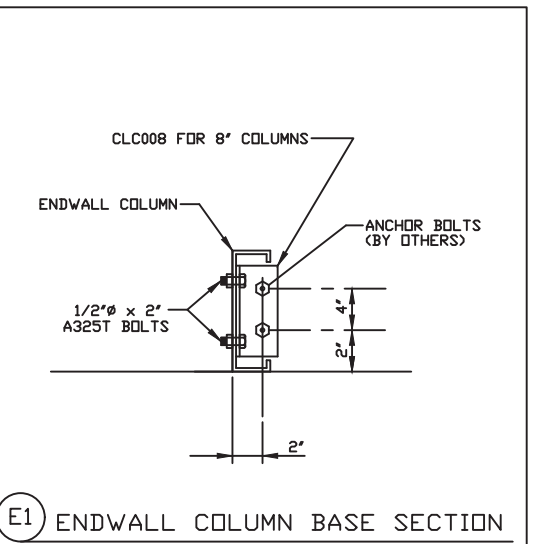
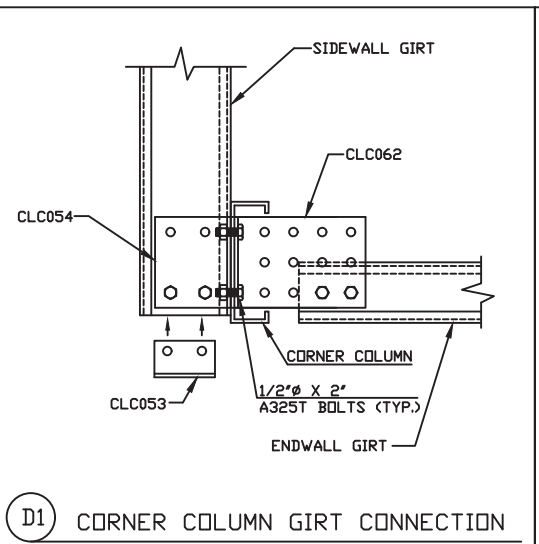
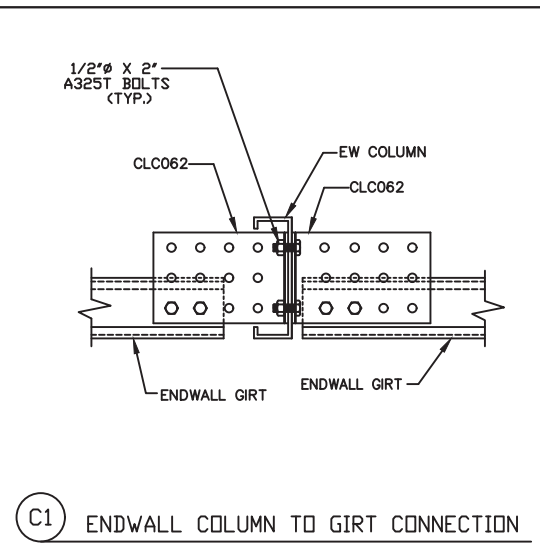
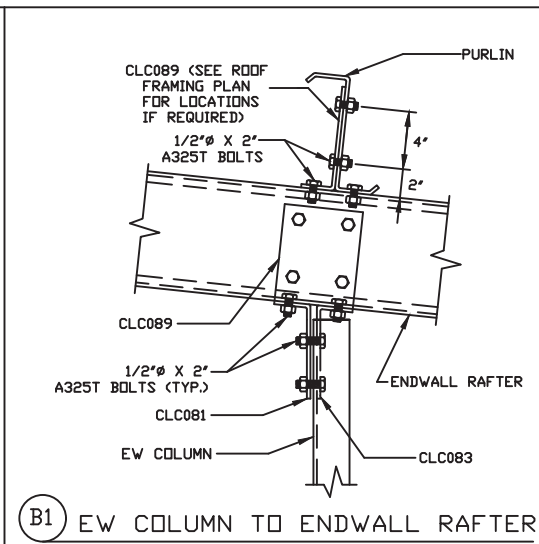
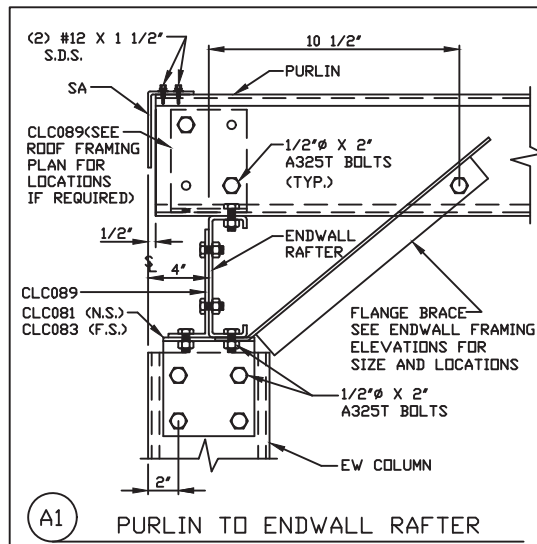
ENDWALL SHEETING & TRIM: FRAME LINE 4  
PANELS: 26 Ga. PBR - Slate Gray



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Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
ENDWALL SHEETING			Sht E8 of 11



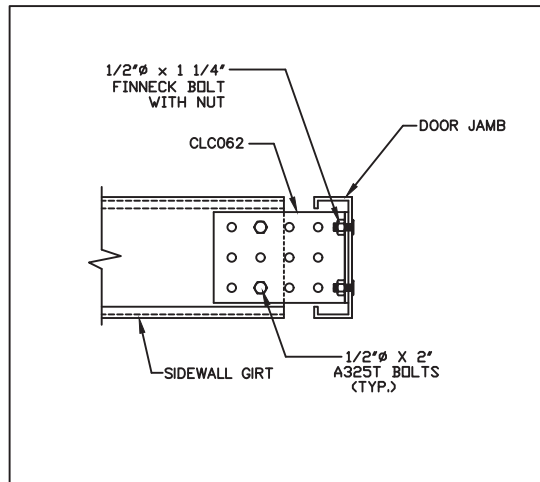
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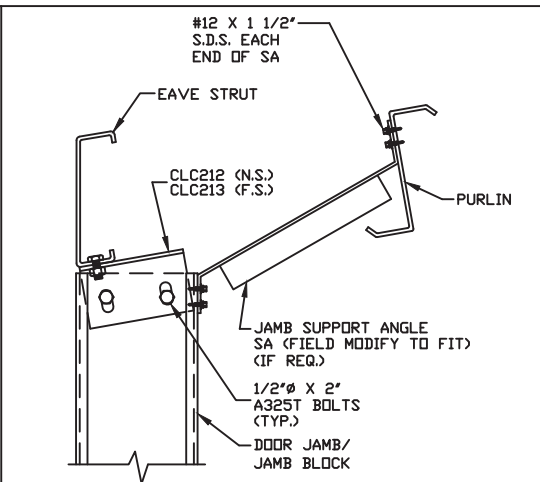
City of Ottawa | Ville d'Ottawa  
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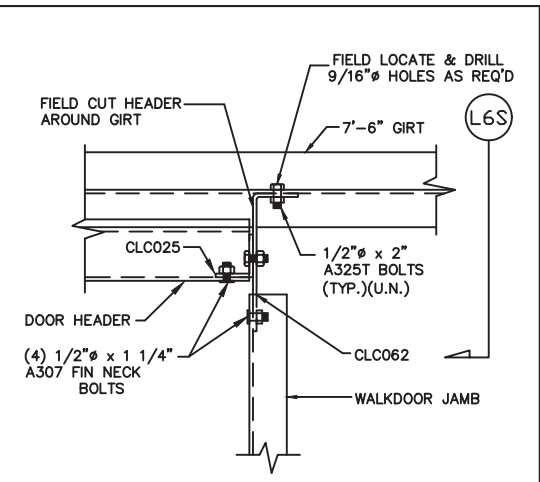
Olympia Steel Buildings Canada		Customer: Clint Millar-Langlois	
Markham ON L3R 0E1		Gloucester ON K1V 1H4	
Drafter: SF	Date: 2/27/24	Designer: MQZ	Date: 2/27/24
Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
DETAIL DRAWINGS			Sht E9 of 11



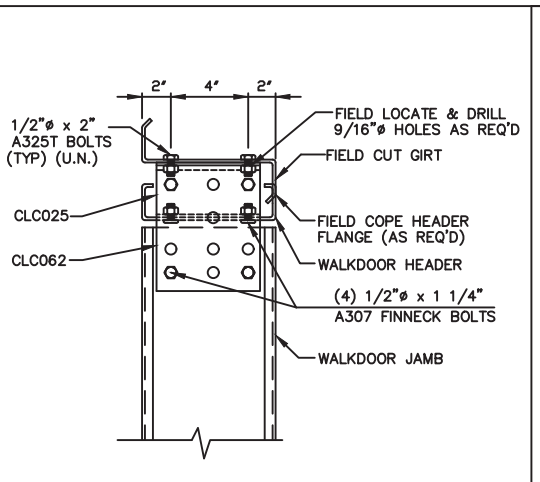
K2 DOOR JAMB TO GIRT CONNECTION



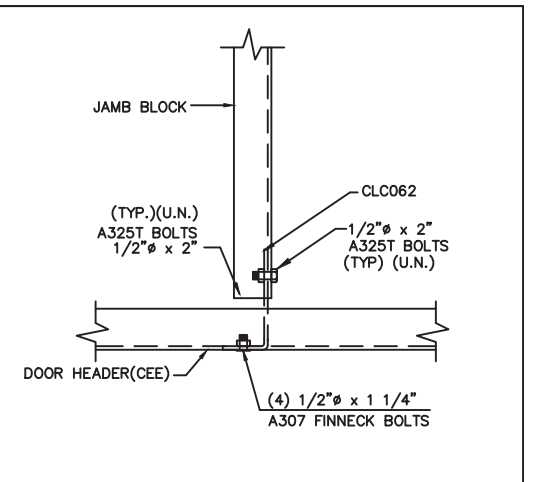
L1 DOOR JAMB/JB BLOCK TO EAVE STRUT



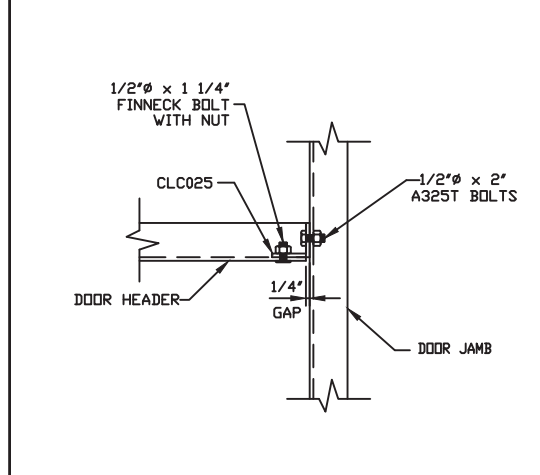
L6 DOOR JAMB & HEADER TO WALL GIRT



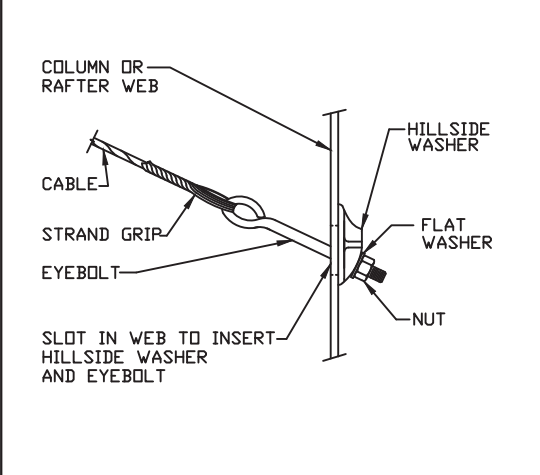
L6S DOOR JAMB & HEADER TO WALL GIRT



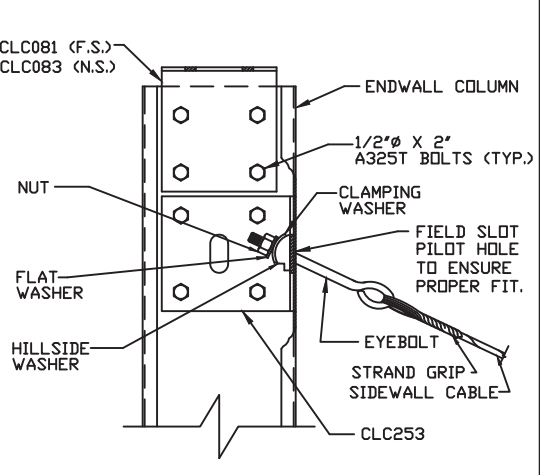
L7 JAMB BLOCK TO HEADER



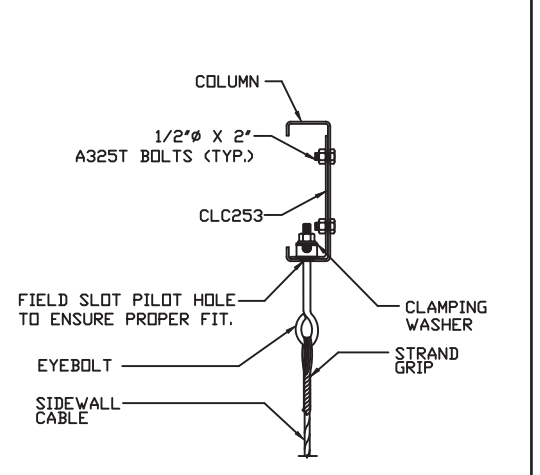
M1 HEADER TO DOOR JAMB CONNECTION



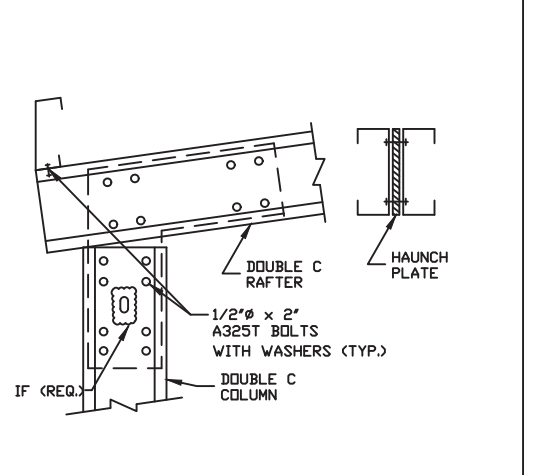
Q2 DIAGONAL CABLE BRACE CONNECTION



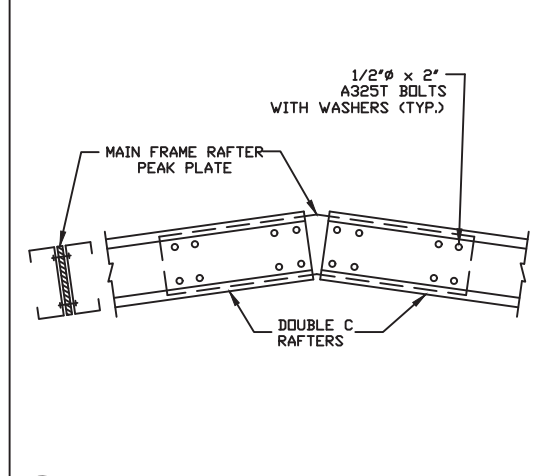
Q8 CABLE TO ENDWALL COLUMN



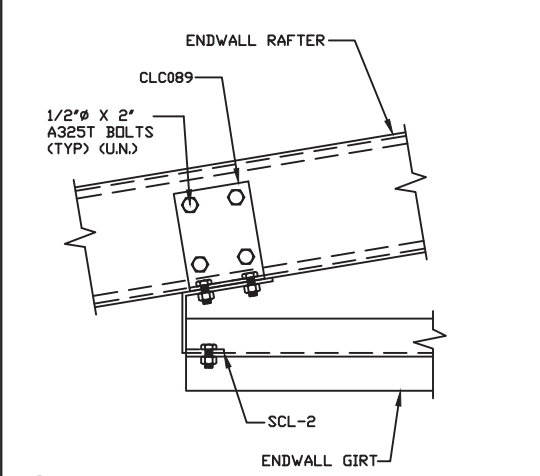
Q9 DIAGONAL CABLE BRACE CONNECTION



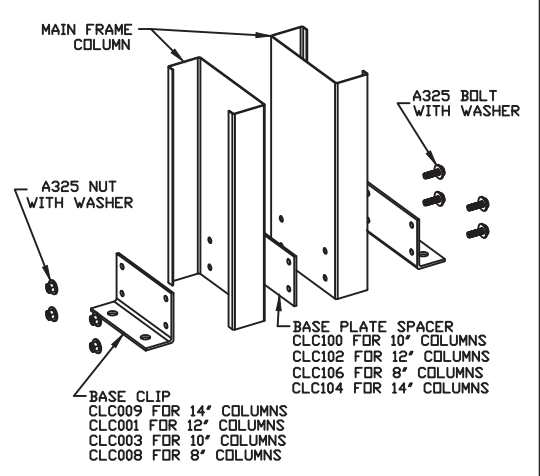
U11 MAIN FRAME COLUMN TO MAIN FRAME RAFTER



U12 MAIN FRAME RAFTER PEAK CONNECTION



W1 ENDWALL GIRT TO ENDWALL RAFTER



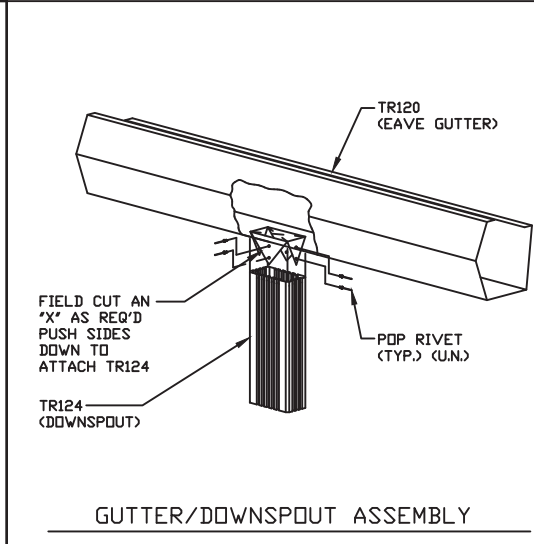
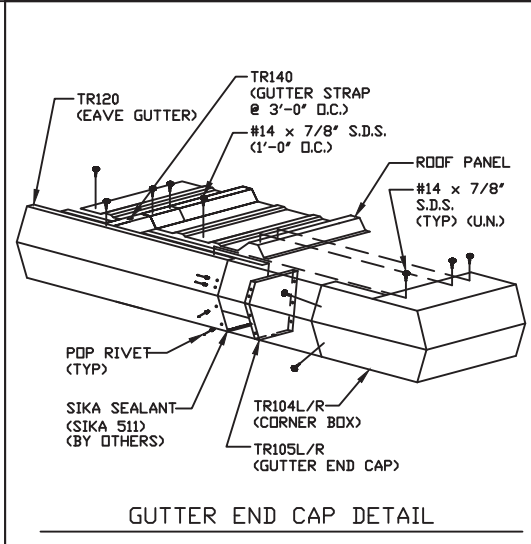
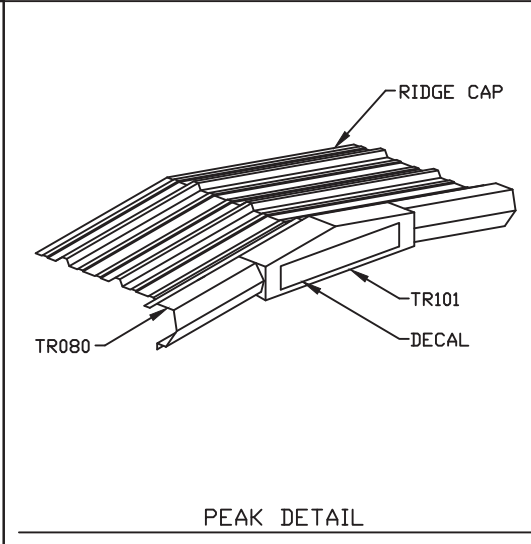
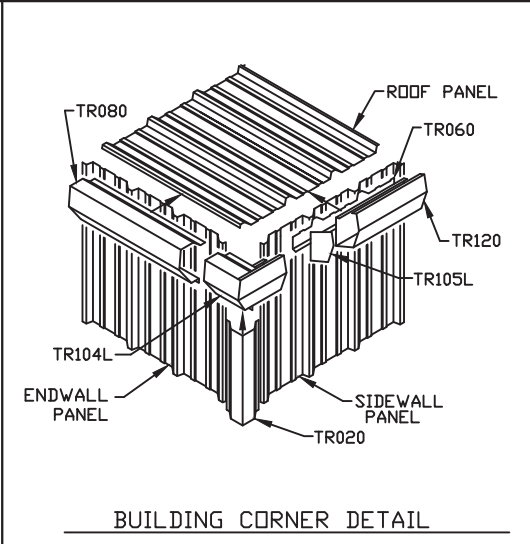
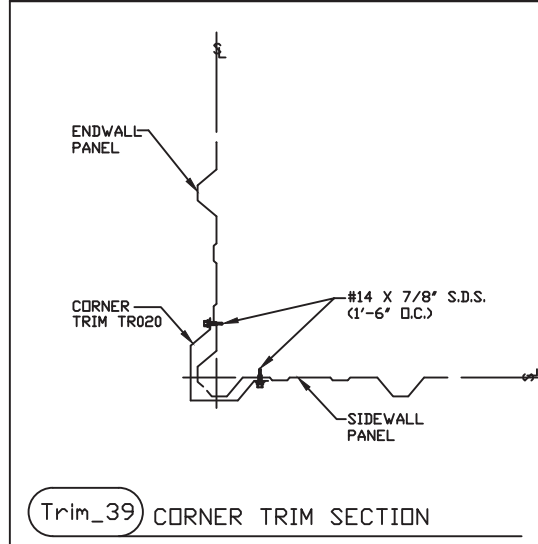
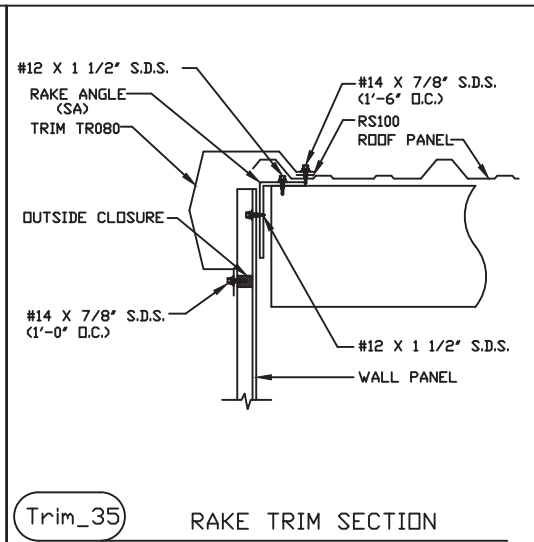
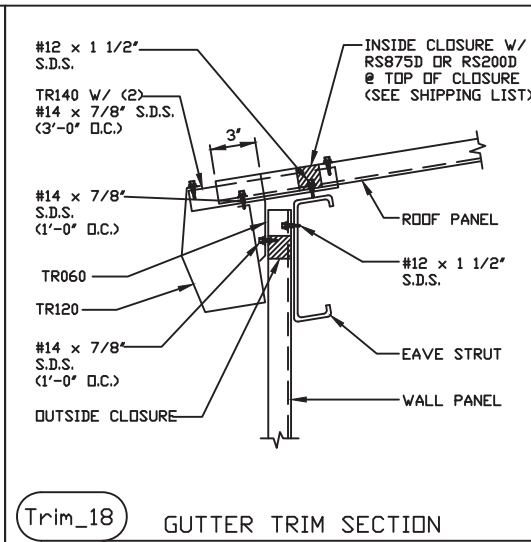
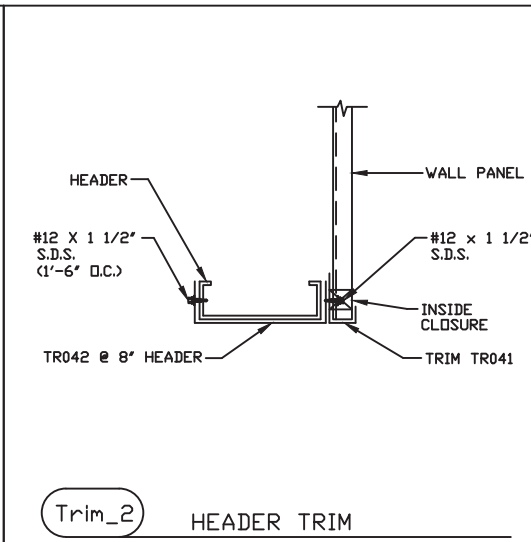
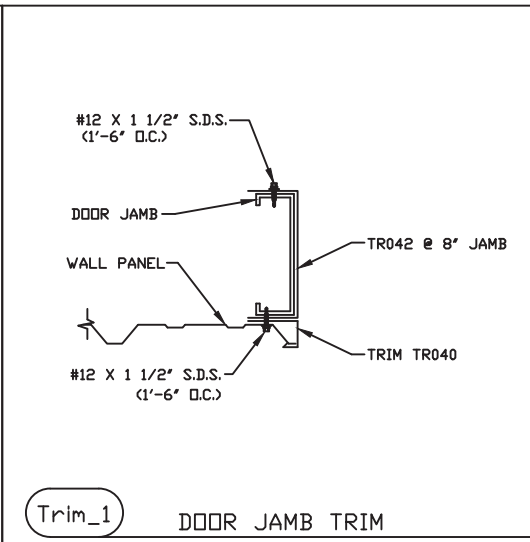
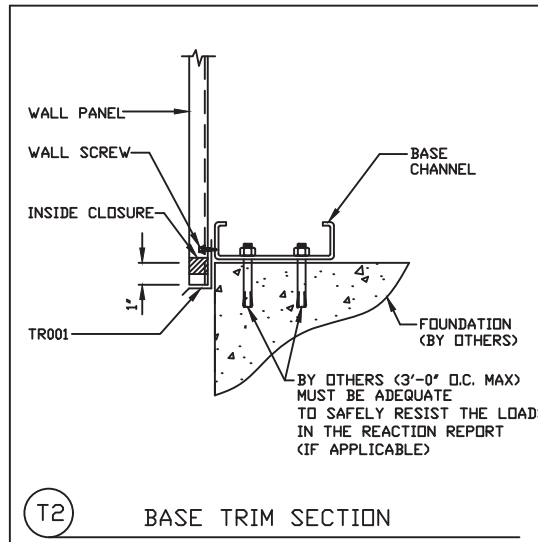
SIDEWALL COLUMN BASE CONNECTION

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Detailer: SSN	Date: 2/27/24	Sales ID:	Factory ID:
Checker: AS	Date: 2/27/24		011353
DETAIL DRAWINGS			Sht E10 of 11

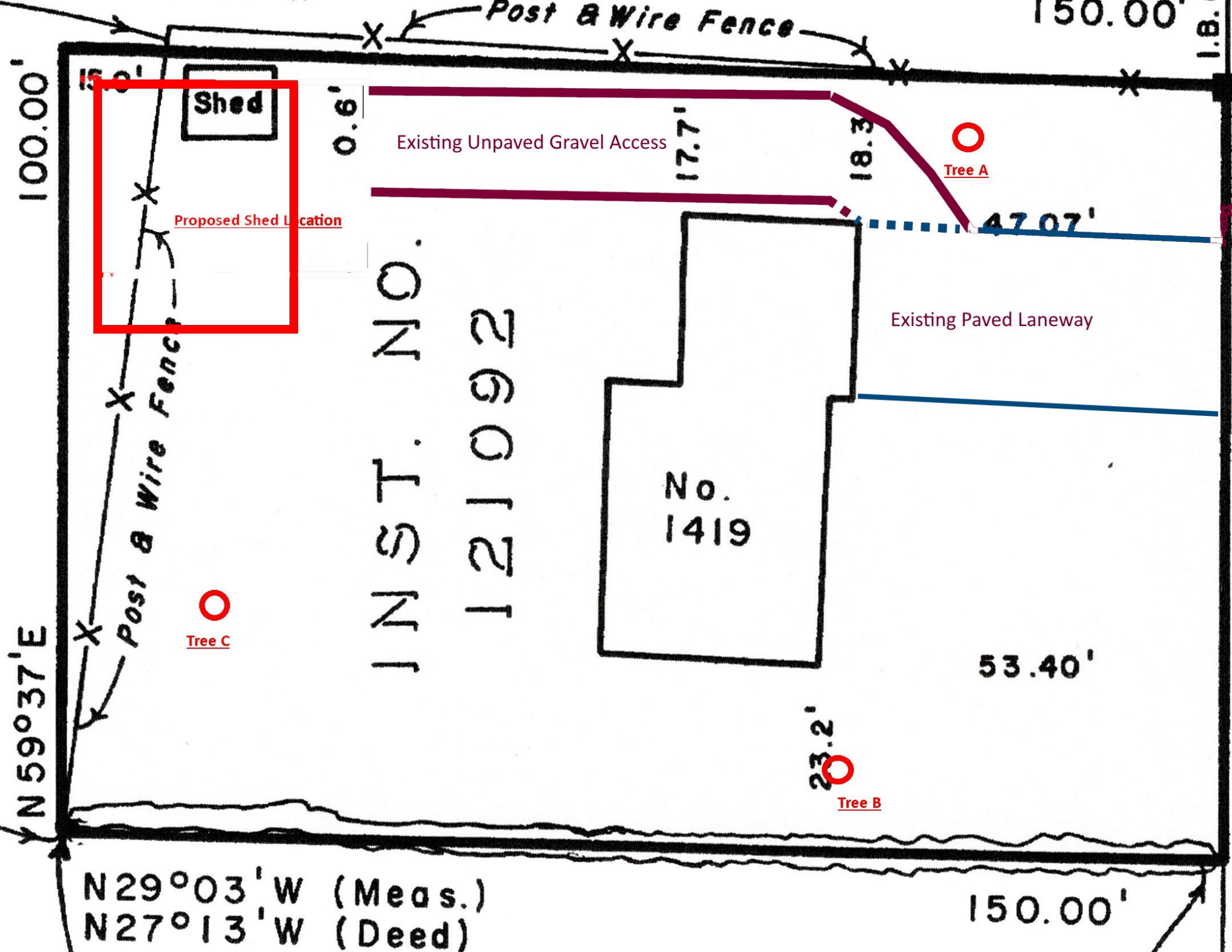


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Checker: AS	Date: 2/27/24		011353
TRIM DRAWINGS			Sht E11 of 11



100.00'

150.00'

1.8.1

Post & Wire Fence

Shed

Proposed Shed Location

Existing Unpaved Gravel Access

Tree A

Existing Paved Laneway

INST. NO.  
121092

No.  
1419

Tree C

Tree B

53.40'

23.2'

N 59° 37' E

N 29° 03' W (Meas.)

N 27° 13' W (Deed)

150.00'