

Attachment A - Scope of Work

Scope of Work

The following section outlines the scope of work completed in regards to the LID Demonstration Projects in accordance with the Work Plan as established in consultation with the City of Ottawa. The established Work Plan has been organized into three (3) distinct phases that are detailed below. It is acknowledged that the work plan will be subject to refinement as part of the review and consultation process with the City and the SWM Working Group.

Phase 1 – Development of Existing Conditions Report and LID Demonstration Project Scoping Document.

Phase 2 - Modelling and Interim Review of Draft Preliminary Design "Demonstration Plan" and includes the following sub-tasks:

- LID Demonstration Plan targets and criteria (water quality, erosion, flood control and infiltration)
 - § Targets for LID Demonstration Plan lot-level controls in regards to the ultimate land-

- uses (i.e. residential vs. employment uses)
- § Targets LID Demonstration Plan for conveyance controls
- § Targets for LID Demonstration Plan end-of-pipe controls to be potentially implemented in future phases based on monitoring results.
- Model selection and agreement on model parameters/inputs for the LID Demonstration Project in consultation with the SWM Working Group
- Modelling, results, analysis and reporting
- Operations & Maintenance and Assumption Protocols
 - § Potential LID operations and maintenance considerations, typical requirements and expected costs.
 - § Recommended LID assumption protocols

- Strategies for subsequent phases and demonstration plans
- Implications for CDP

Phase 3 - Preliminary Design for LID Demonstration Project Phase 1 and includes the following sub-tasks:

- o Final Review of Preliminary Design
- CDP Recommendations
- Implementation recommendations including but not limited to:
 - Monitoring program recommendations based on project objectives and targets developed from Stage 1
 - LID Demonstration Project reporting requirements
 - Process (feedback process) whereby monitoring results are used to inform/refine future LID phases and used to refine modelling assumptions and SWM credits.

Phase 2 – Specifics

The following section provides greater detail in regards to the specific activities completed in the satisfaction of Phase 2 of the Work Plan.

1. Infiltration Testing

Infiltration testing of surficial soils will be completed to carry out the LID feasibility analysis and future design. The proposed Work Plan is based on borehole and test pit logs completed by DST. The in-situ infiltration testing is required for all infiltration-based LID designs and must be used to confirm and to refine the coarse estimates performed as part of the geotechnical assessment. The testing is required to determine the infiltration capacity of site specific soils, to design appropriate sizes of each facility, to determine if underdrains are required and to locate any drains within the facility crosssection. These data will allow an accurate forecast of post-development performance.

In-situ soil testing will be a combination of:

 Guelph Permeameter Testing apparatus and protocols to determine the in-situ saturated hydraulic conductivity and the design infiltration rate as per the LID Stormwater Planning and Design Guide Version 1.0 (TRCA/CVC 2010). Testing locations will be targeted to the proposed footprint of the infiltration facilities as required. Approximately 22 locations are proposed for sampling.

 Double Ring Infiltrometer to provide an infiltration rate of shallow soils at the existing land surface, at the likely interface of the proposed grade raise, to account for the effects of micro-tubules, rootlets and other macropores. A total of 6 tests are anticipated.

2. Water Balance Model

The development of a water balance model for the Former CFB Rockcliffe CDP site will be completed using EPA-SWMM software, and will be based on the completed geotechnical and hydrogeological reports.

The model will be developed such that future (Stage 2 and beyond) LID options can be assessed in terms of flow reductions (water quantity and erosion), water quality, and infiltration. The model results will be utilized to develop LID SWM targets. It is

assumed that the model would include Phases 1-3 of the Rockcliffe Development.

The water balance assessment of the study area will analyze pre- and post-development hydrology and carry out an impact assessment using the following two methods:

- Assessment of the hydrologic regime: long term hydrologic modeling to assess the overall hydrology of the study area,
- Assessment of Infiltration Deficit: specific assessment for estimating the difference between water infiltrated into the ground under pre- and postdevelopment conditions.
- 2.1 Assessment of the Hydrologic Regime
 Long term hydrologic modeling for pre- and
 post-development conditions will be carried
 out to define general hydrologic processes,
 specifically surface runoff hydrology (flow
 rate and runoff volume) and infiltration and
 evaporation volumes. Since observed
 streamflow data are not available to
 calibrate and validate the hydrologic model,
 the model will not be calibrated. However,
 available background information from the

study area, in addition to previous modeling experience under similar conditions, will be used to refine the results as appropriate.

To develop the long term hydrologic model, a continuous precipitation and air temperature record has been gathered and will be applied.

The EPA SWMM model was used as a modeling platform. Developed by the U.S. Environmental Protection Agency (EPA); the EPA SWMM model and its variants (XPSWMM, PCSWMM, InfoSWMM) is a widely used model that is well suited for urban and rural areas, as it performs water quality, quantity and water balance assessments suitable for LID simulations. The model was successfully applied by Aquafor as part of subwatershed studies, stormwater management master plans, and site-based LID modeling and analysis.

2.2 Assessment of Infiltration Deficit

The assessment of the impact of development on infiltration to the ground will be done in accordance with Hydrogeological Assessment Submissions-Conservation Authority Guidelines (June,

2013). Accordingly, the following activities will be undertaken:

- a) Collect climate data (air temperature), soils data (soil types and hydraulic conductivity), and land use data (imperviousness and land cover) for the study area;
- Estimate the evapotranspiration component of the hydrologic regime for the Study Area using Thornthwaite (1948) and available climate data, and calculate total water surplus;
- Define catchment area/Management Unit area, including imperviousness, land cover, and runoff coefficient under pre- and post-development conditions;
- d) Pre-development assessment: Estimate inputs (including precipitation, run-on and other inputs) and outputs (including evapotranspiration, infiltration, and surface runoff).
- e) Post-development assessment: Estimate inputs and outputs.
- f) Estimate the output volume deficit between pre- and post-development (i.e. changes in hydrologic output volumes between pre- and postdevelopment scenarios).

For the assessment of infiltration deficit, water balance spreadsheets (Microsoft Excel) will be used, and output volumes will be estimated using imperviousness values and other physical parameters deemed to change under post-development conditions for different land uses and catchment areas.

- 3. Stream Erosion Assessment
 The erosion assessment scenarios would include the following:
- a) Pre-development conditions scenario
- b) Post-development conditions scenario (including all proposed development) without SWM control
- c) Post-development conditions scenario (including all proposed development)
 with SWM control

In order to run these scenarios, a long term hydrologic model will be carried out and statistical analyses will be done to estimate the change in time of exceedance (flow duration analysis) for storm events with different frequencies under pre- and post-development conditions. The flow regimes (magnitudes and frequencies) will be analyzed under pre- and post-development conditions, and comparative results will be

presented in figures and tables for the two flow regimes.

Moreover, the 2-year storm event, which is generally linked to bankfull flows responsible for sediment removal and channel maintenance, will be analyzed under pre- and post-development conditions (in addition to post-development with SWM control conditions). All relevant pond design information (including size, surface area, and control structures) will be included in the model.

It should be noted that the hydraulic assessment of stream erosion under preand post-development conditions will not be included at this stage. More specifically, developing a hydraulic model (HEC-RAS) for the two streams should be a subsequent step (see Step 5) that would use the hydrologic model input to investigate hydraulic variables including shear stress, velocity, and water depth along the two streams. Accordingly, key locations such as downstream of culverts and the potential impact of increasing flows on channel stability would be further investigated and analyzed.

- 4. Hydraulic Model Development
 A detailed hydraulic modelling of the Eastern
 and Western tributaries will be developed to
 facilitate stream rehabilitation assessments
 and designs using the HEC-RAS modelling
 software.
- Operations & Maintenance and Assumption Protocols
 The completed tasks will be completed in parallel to the MSS including but not limited to:
 - a) Potential LID operations and maintenance considerations, typical requirements and expected costs.
 - b) Recommended LID assumption protocols
- 6. Strategies for Subsequent Phases and Demonstration PlansIn consultation with the City of Ottawa and the SWM Working Group strategies for each

the SWM Working Group strategies for each subsequent phase of development and servicing will be developed including plans for subsequent phases of the LID Demonstration Project

Phase 3 - Specifics

In the fulfillment of Phase 3 of the Work Plan, the following tasks were completed.

- 1. Development of Typical LID Details
 Following the selection of the preferred LID
 controls for each land-use, typical LID design
 details will be prepared (i.e. preliminary
 design) in support of the CDP. LID
 Preliminary concepts will include:
 - a) Location of proposed LID,
 - b) Types of LIDs,
 - c) Targets and design criteria,
 - d) Anticipated performance of LIDs determined through modelling using the previously developed EPA-SWMM model (Phase 2).

2. LID Recommendations

At the conclusion of Phase 3, a comprehensive document will be prepared which will detail the results of Work Plan (Phases 1-3) and will conclude with implementation recommendations including but not limited to:

- a) By-law considerations
- b) Construction sequencing recommendations, protocols and erosion and sediment control requirements.

- c) Reporting considerations for privately owned facilities
- d) Assumption, operation and maintenance protocols (typical tasks, frequency and responsible party)
- e) Inspection recommendations

- f) Monitoring program recommendations based on project objectives and targets developed from Phase 2
- g) LID demonstration project reporting requirements
- h) Adaptive Management Process (feedback process) whereby

monitoring results are used to inform/refine future LID phases and used to refine modelling assumptions and SWM credits.

Aquafor Beech Ltd. February, 2015

Attachment B – Hydrologic Model (Pre-Development)

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
    ****************
Analysis Options
Flow Units ..... CMS
Process Models:
 Rainfall/Runoff ..... YES
  Snowmelt ...... NO
  Groundwater ..... NO
 Flow Routing ..... YES Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... GREEN_AMPT
Flow Routing Method ..... KINWAVE
Starting Date ..... AUG-01-1996 00:00:00
Ending Date ..... JAN-01-2014 06:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ...... 00:15:00
Wet Time Step ..... 00:15:00
Dry Time Step ..... 01:00:00
Routing Time Step ...... 30.00 sec
******
Rainfall File Summary
*******
Station First Last Recording Periods Periods Periods ID Date Date Frequency w/Precip Missing Malfunc.
        AUG-08-1996 DEC-31-2013 60 min 28488
                                     Depth
*******
                          Volume
Runoff Quantity Continuity hectare-m
                                     mm
*****
******
                          Volume
                                      Volume
                                   10^6 ltr
Flow Routing Continuity
                         hectare-m
Dry Weather Inflow .....
                            0.000
                                       0.000
                        992.447
                                   9924.573
Wet Weather Inflow .....
Groundwater Inflow .....
                                    0.000
                         0.000
0.000
0.000
RDII Inflow .....
                                      0.000
External Inflow .....
                        0.000
984.057
                                    9840.669
External Outflow .....
                          9.336
                                    93.357
Internal Outflow ......
Storage Losses ......
Initial Stored Volume ....
                           0.000
                                      0.000
Final Stored Volume .....
Final Stored Volume ..... 0.000
Continuity Error (%) ..... -0.095
```

All links are stable.

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 1.01

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	15454.66	0.00	423.97	12215.04	2853.71	410.37	1.14	0.185
EXT2	15454.66	0.00	978.90	7619.12	6939.50	204.72	0.28	0.449
BRSWM1	15454.66	0.00	1059.20	7627.17	6853.37	723.72	1.00	0.443
BRSWM2	15454.66	19250.81	1707.07	8318.19	24837.39	1515.09	2.10	0.716
BRSWM3	15454.66	0.00	1176.44	7019.39	7348.56	1174.31	1.53	0.475
EXT3	15454.66	0.00	429.51	12219.06	2843.68	522.96	1.42	0.184
EXTW	15454.66	0.00	225.91	13801.71	1446.36	266.28	0.88	0.094
EXN	15454.66	0.00	830.11	9724.26	4969.94	1806.09	3.42	0.322
EXTN	15454.66	0.00	292.53	13568.94	1625.97	387.31	1.79	0.105
EXW1	15454.66	4003.33	1239.31	9215.03	9082.42	6730.11	10.83	0.467
LIDPilot	15454.66	0.00	454.98	12239.70	2796.35	324.38	0.74	0.181

Node Depth Summary

Node	Туре	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Occu	of Max errence hr:min
2	JUNCTION	0.02	0.70	1.00	7	14:30
1	JUNCTION	0.02	0.70	1.70	7	14:23
5	JUNCTION	0.00	0.31	5.31	3623	17:00
6	JUNCTION	0.00	0.39	3.39	3623	17:00
WesternOutfall	OUTFALL	0.01	0.64	0.64	3653	22:00
EasternOutfall	OUTFALL	0.00	0.39	1.39	3623	17:00

Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	0ccur:	rence	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL	0.883 12.679 3.416 1.788 0.000	5.521 12.679 3.416 5.205 5.520	3623 3623 3623 3653	17:00 17:00 17:00 22:00	266.277 7464.858 1806.085 387.307 0.000	7647.375 7464.858 1806.085 2193.094 7647.500
OUTFALL	0.000	5.192	3623	17:00	0.000	2193.124
	JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL	Lateral Inflow CMS JUNCTION 0.883 JUNCTION 12.679 JUNCTION 3.416 JUNCTION 1.788 OUTFALL 0.000	Lateral Inflow Inflow CMS Total Inflow Inflow CMS JUNCTION 0.883 JUNCTION 12.679 JUNCTION 3.416 JUNCTION 1.788 JUNCTION 1.788 JUNCTION 0.000 5.520	Lateral Total Time o	Lateral Total Time of Max Inflow Inflow Occurrence Occurrence CMS CMS days hr:min	Lateral Total Time of Max Inflow Inflow Occurrence Inflow Volume 10^6 ltr Type CMS CMS days hr:min days hr:min 10^6 ltr JUNCTION 0.883 5.521 3653 22:00 266.277 JUNCTION 12.679 12.679 3623 17:00 7464.858 JUNCTION 3.416 3.416 3623 17:00 1806.085 JUNCTION 1.788 5.205 3623 17:00 387.307 OUTFALL 0.000 5.520 3653 22:00 0.000

Surcharging occurs when water rises above the top of the highest conduit.

Node	Туре	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2	JUNCTION JUNCTION	8.94 10.19	0.000	0.000

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m3
1	10.19	8.009	3623 17:00	93.356	0.000

	Flow	Avg.	Max.	Total
	Freq.	Flow	Flow	Volume
Outfall Node	Pcnt.	CMS	CMS	10 ^ 6 ltr
WesternOutfall	27.64	0.050	5.520	7647.500
EasternOutfall	20.36	0.020	5.192	2193.124
System	24.00	0.070	10.512	9840.624

Link	Туре	Maximum Flow CMS	Time of Occur days h	rence	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2 Eastern1	CHANNEL CHANNEL	5.520 3.418		22:00 17:00	2.81 7.16	0.91 0.06	0.92 0.24
Eastern2 Western1	CHANNEL CHANNEL	5.192 4.765		17:00 15:59	8.04	0.09	0.30

Conduit		Hours Full Upstream		Hours Above Full Normal Flow	
Western1	8.67	10.09	8.94	10.21	10.09

Analysis begun on: Wed Feb 18 17:14:20 2015

Analysis ended on: Wed Feb 18 17:22:14 2015 Total elapsed time: 00:07:54

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NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units CMS Process Models: Rainfall/Runoff YES Snowmelt NO Groundwater NO Flow Routing YES Ponding Allowed NO

Water Quality NO
Infiltration Method GREEN_AMPT

Flow Routing Method KINWAVE

Starting Date AUG-01-1996 00:00:00 Ending Date JAN-01-2014 06:00:00

Antecedent Dry Days 0.0 Report Time Step 00:15:00 Wet Time Step 00:15:00 Dry Time Step 01:00:00
Routing Time Step 30.00 sec

******* Rainfall File Summary

Station First Last Recording Periods Periods Periods ID Date Date Frequency w/Precip Missing Malfunc. 1 AUG-08-1996 DEC-31-2013 60 min 28488 0

*******	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm

Total Precipitation	3595.989	15454.656
Evaporation Loss	170.899	734.482
Infiltration Loss	2953.427	12693.086
Surface Runoff	486.490	2090.810
Final Surface Storage	0.040	0.171
Continuity Error (%)	-0.413	
*******	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr

Dry Weather Inflow	0.000	0.000

 Dry Weather Inflow
 0.000
 0.000

 Wet Weather Inflow
 486.490
 4864.948

 Groundwater Inflow
 0.000
 0.000

 RDII Inflow
 0.000
 0.000

 External Inflow
 0.000
 0.000

 External Outflow
 482.152
 4821.574

 Internal Outflow
 4.873
 48.726

 Storage Losses
 0.000
 0.000

 Initial Stored Volume
 0.000
 0.000

 Final Stored Volume
 0.000
 0.000

 Continuity Error (%)
 -0.110

 -0.110 Continuity Error (%)

******* Highest Flow Instability Indexes *********

All links are stable.

******** Routing Time Step Summary ******

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00

	Total	Total	Total	Total	Total	Total	Peak	Runoff
Subcatchment	Precip mm	Runon mm	Evap mm	Infil mm	Runoff mm	Runoff 10^6 ltr	Runoff CMS	Coeff
EXT1	15454.66	0.00	610.19	12215.04	2687.48	386.46	1.14	0.174
EXT2	15454.66	0.00	1456.21	7619.12	6524.68	192.48	0.28	0.422
BRSWM1	15454.66	0.00	1514.30	7627.17	6434.81	679.52	1.00	0.416
BRSWM2	15454.66	16771.03	1630.25	14953.71	15836.52	966.03	2.01	0.491
BRSWM3	15454.66	0.00	1533.71	7630.58	6401.96	1023.04	1.50	0.414
EXT3	15454.66	0.00	614.36	12219.06	2676.97	492.30	1.42	0.173
EXW	15454.66	2717.57	654.27	14247.23	3320.43	2847.29	9.11	0.183
EXTW	15454.66	0.00	319.77	13801.71	1363.37	251.00	0.88	0.088
EXN	15454.66	0.00	683.89	12049.48	2786.52	1012.63	3.13	0.180
EXTN	15454.66	0.00	388.06	13569.49	1543.03	367.55	1.79	0.100

Node	Туре	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Occi	of Max urrence hr:min
2	JUNCTION	0.01	0.70	1.00	7	14:59
1	JUNCTION	0.01	0.70	1.70	7	14:53
5	JUNCTION	0.00	0.30	5.30	3623	17:00
6	JUNCTION	0.00	0.38	3.38	3623	17:00
WesternOutfall	OUTFALL	0.01	0.64	0.64	3653	22:00
EasternOutfall	OUTFALL	0.00	0.38	1.38	3623	17:00

Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Ma Occurrence days hr:m:	ce Volume	Total Inflow Volume 10^6 ltr
2	JUNCTION	0.883	5.521	3653 22:	00 250.998	3441.516
1	JUNCTION	10.224	10.224	3653 21:0	3233.750	3233.750
5	JUNCTION	3.130	3.130	3623 17:0	00 1012.627	1012.627
6	JUNCTION	1.788	4.915	3623 17:	367.551	1379.962
WesternOutfall	OUTFALL	0.000	5.520	3653 22:	0.000	3441.605
EasternOutfall	OUTFALL	0.000	4.911	3623 17:0	0.000	1379.947

Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Туре	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2 1	JUNCTION JUNCTION	5.53 5.90	0.000	0.000

Flooding refers to all water that overflows a node, whether it ponds or not.

Total Maximum Maximum Time of Max Flood Ponded Hours Rate Occurrence Volume Volume

Node	Flooded	CMS	days hr:min	10 ^ 6 ltr	1000 m3
1	5.90	5.578	3653 21:00	48.725	0.000

Outfall Loading Summary *********

Outfall Node	Flow	Avg.	Max.	Total
	Freq.	Flow	Flow	Volume
	Pcnt.	CMS	CMS	10^6 ltr
WesternOutfall	24.74	0.025	5.520	3441.605
EasternOutfall	19.53	0.013	4.911	1379.947
System	22.14	0.038	10.230	4821.552

****** Link Flow Summary

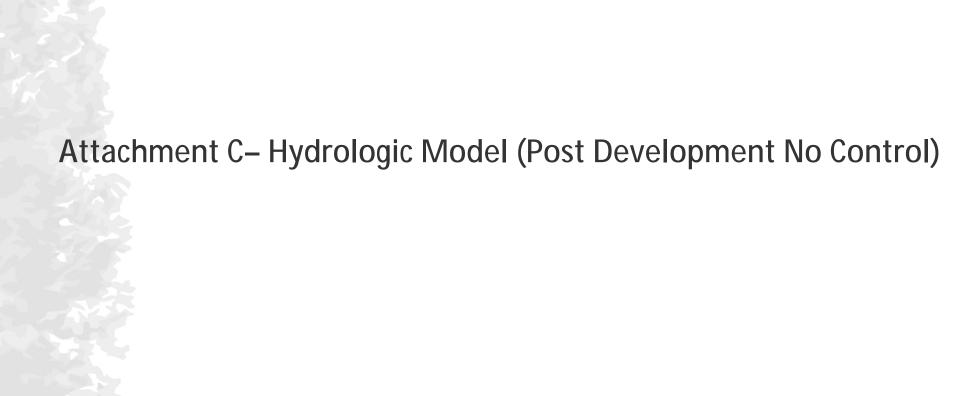
Link	Туре	Maximum Flow CMS	0ccu	of Max rrence hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2 Eastern1 Eastern2 Western1	CHANNEL	5.520	3653	22:00	2.81	0.91	0.92
	CHANNEL	3.128	3623	17:00	6.97	0.06	0.23
	CHANNEL	4.911	3623	17:00	7.91	0.09	0.29
	CHANNEL	4.763	2961	11:16	2.30	1.03	1.00

Conduit Surcharge Summary

Conduit		Hours Full Upstream		Hours Above Full Normal Flow	Hours Capacity Limited
Western1	5.41	5.86	5.53	5.94	5.86

Analysis begun on: Wed Feb 18 17:03:20 2015 Analysis ended on: Wed Feb 18 17:11:11 2015 Total elapsed time: 00:07:51

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
    ****************
Analysis Options
Flow Units ..... CMS
Process Models:
  Rainfall/Runoff ..... YES
  Snowmelt ...... NO
  Groundwater ..... NO
  Flow Routing ..... YES Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... GREEN_AMPT
Flow Routing Method ..... KINWAVE
Starting Date ..... AUG-01-1996 00:00:00
Ending Date ..... JAN-01-2014 06:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ...... 00:15:00
Wet Time Step ..... 00:15:00
Dry Time Step ..... 01:00:00
Routing Time Step ...... 30.00 sec
******
Rainfall File Summary
*******
Station First Last Recording Periods Periods Periods ID Date Date Frequency w/Precip Missing Malfunc.
         AUG-08-1996 DEC-31-2013 60 min 28488
                                           Depth
*******
                               Volume
Runoff Quantity Continuity hectare-m
                                          mm
Total Precipitation ... 3595.217 15454.656
Evaporation Loss ... 200.885 863.539
Infiltration Loss ... 2388.009 10265.265
Surface Runoff ... 1021.296 4390.215
Final Surface Storage 0.004 0.019
Continuity Error (%) -0.417
******
                              Volume
                                             Volume
                                          10^6 ltr
Flow Routing Continuity
                             hectare-m
*********
Dry Weather Inflow .....
                                0.000
                                             0.000
                                        10213.064
Groundwater Inflow ......
RDII Inflow
                              1021.296
                                          0.000
                             0.000
0.000
0.000
RDII Inflow .....
                                             0.000
External Inflow .....
                           0.000 0.000
1012.441 10124.514
9.813 98.132
0.000 0.000
External Outflow .....
Internal Outflow ......
Storage Losses ......
                               0.000
Initial Stored Volume ....
                                             0.000
Final Stored Volume .....
Continuity Error (%) ..... 0.000
```

All links are stable.

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 1.01

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	15454.66	0.00	423.97	12215.04	2853.71	410.37	1.14	0.185
EXT2	15454.66	0.00	978.90	7619.12	6939.50	204.72	0.28	0.449
BRSWM1	15454.66	0.00	1059.20	7627.17	6853.37	723.72	1.00	0.443
BRSWM2	15454.66	19250.81	1707.07	8318.19	24837.39	1515.09	2.10	0.716
BRSWM3	15454.66	0.00	1176.44	7019.39	7348.56	1174.31	1.53	0.475
EXT3	15454.66	0.00	429.51	12219.06	2843.68	522.96	1.42	0.184
EXTW	15454.66	0.00	225.91	13801.71	1446.36	266.28	0.88	0.094
EXN	15454.66	0.00	830.11	9724.26	4969.94	1806.09	3.42	0.322
EXTN	15454.66	0.00	292.53	13568.94	1625.97	387.31	1.79	0.105
EXW1	15454.66	4003.33	1239.31	9215.03	9082.42	6730.11	10.83	0.467
LIDPilot	15454.66	0.00	903.31	9327.81	5283.31	612.87	0.90	0.342

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	0ccu	of Max rrence hr:min
2	JUNCTION	0.02	0.70	1.00	7	14:27
1	JUNCTION	0.02	0.70	1.70	7	14:20
5	JUNCTION	0.00	0.31	5.31	3623	17:00
6	JUNCTION	0.00	0.39	3.39	3623	17:00
WesternOutfall	OUTFALL	0.02	0.64	0.64	3653	22:00
EasternOutfall	OUTFALL	0.00	0.39	1.39	3623	17:00

Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	0ccu	of Max rrence hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
2 1 5 6 WesternOutfall EasternOutfall	JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL OUTFALL	0.883 12.871 3.416 1.788 0.000 0.000	5.521 12.871 3.416 5.205 5.520	3653 3623 3623 3623 3653 3623	22:00 17:00 17:00 17:00 22:00	266.277 7753.348 1806.085 387.307 0.000	7931.218 7753.348 1806.085 2193.094 7931.344 2193.124

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2	JUNCTION JUNCTION	9.50 10.72	0.000	0.000

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m3
1	10.72	8.202	3623 17:00	98.132	0.000

	Flow Freq.	Avg. Flow	Max. Flow	Total Volume
Outfall Node	Pcnt.	CMS	CMS	10 ^ 6 ltr
WesternOutfall EasternOutfall	27.77 20.36	0.052 0.020	5.520 5.192	7931.344 2193.124
System	24.07	0.072	10.512	10124.467

Link	Туре	Maximum Flow CMS	0ccu	of Max rrence hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2	CHANNEL	5.520	3653	22:00	2.81	0.91	0.92
Eastern1	CHANNEL	3.418	3623	17:00	7.16		0.24
Eastern2	CHANNEL	5.192	3623	17:00	8.04	0.09	0.30
Western1	CHANNEL	4.765	5870	13:05	2.38	1.03	

Conduit		Hours Full Upstream		Hours Above Full Normal Flow	
Western1	9.21	10.62	9.50	10.76	10.62

Analysis begun on: Wed Feb 18 17:34:21 2015

Analysis ended on: Wed Feb 18 17:42:06 2015 Total elapsed time: 00:07:45

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```
EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
   ****************
Analysis Options
Flow Units ..... CMS
Process Models:
 Rainfall/Runoff ..... YES
 Snowmelt ...... NO
 Groundwater ..... NO
 Flow Routing ..... YES Ponding Allowed ..... NO
 Water Quality ..... NO
Infiltration Method ..... GREEN_AMPT
Flow Routing Method ..... KINWAVE
Starting Date ..... AUG-01-1996 00:00:00
Ending Date ..... JAN-01-2014 06:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ...... 00:15:00
Wet Time Step ..... 00:15:00
Dry Time Step ..... 01:00:00
Routing Time Step ...... 30.00 sec
******
Rainfall File Summary
*******
Station First Last Recording Periods Periods Periods ID Date Date Frequency w/Precip Missing Malfunc.
        AUG-08-1996 DEC-31-2013 60 min 28488
                                     Depth
*******
                          Volume
Runoff Quantity Continuity hectare-m
                                    mm
******
******
                          Volume
                                      Volume
                                    10^6 ltr
Flow Routing Continuity
                         hectare-m
*********
Dry Weather Inflow .....
                           0.000
                                      0.000
                                  10153.517
Groundwater Inflow ..... 1015.341
Groundwater Inflow ..... 0.000
RDII Inflow
                                    0.000
                         0.000
0.000
0.000
                                      0.000
RDII Inflow .....
External Inflow .....
                       0.000 0.000
1006.737 10067.471
External Outflow .....
                         9.529
                                    95.291
0.000
Internal Outflow ......
Storage Losses ......
Initial Stored Volume ....
                           0.000
                                      0.000
Final Stored Volume .....
```

Continuity Error (%) -0.091

All links are stable.

Routing Time Step Summary

Minimum Time Step : 30.00 sec

Average Time Step : 30.00 sec

Maximum Time Step : 30.00 sec

Percent in Steady State : 0.00

Average Iterations per Step : 1.01

******* Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	15454.66	0.00	423.97	12215.04	2853.71	410.37	1.14	0.185
EXT2	15454.66	0.00	978.90	7619.12	6939.50	204.72	0.28	0.449
BRSWM1	15454.66	0.00	1059.20	7627.17	6853.37	723.72	1.00	0.443
BRSWM2	15454.66	19250.81	1707.07	8318.19	24837.39	1515.09	2.10	0.716
BRSWM3	15454.66	0.00	1176.44	7019.39	7348.56	1174.31	1.53	0.475
EXT3	15454.66	0.00	429.51	12219.06	2843.68	522.96	1.42	0.184
EXW	15454.66	3459.44	1205.38	9288.93	8493.75	7283.44	11.57	0.449
EXTW	15454.66	0.00	225.91	13801.71	1446.36	266.28	0.88	0.094
EXN	15454.66	0.00	829.88	9724.21	4969.94	1806.09	3.42	0.322
EXTN	15454.66	0.00	292.64	13568.83	1625.97	387.31	1.79	0.105

****** Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	0ccu	of Max rrence hr:min
2 1 5 6	JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL	0.02 0.02 0.00 0.00	0.70 0.70 0.31 0.39	1.00 1.70 5.31 3.39	7 7 3623 3623 3653	14:28 14:21 17:00 17:00 22:00
WesternOutfall EasternOutfall	OUTFALL	0.02	0.64	1.39	3623	17:00

Node Inflow Summary ********

Node	Туре	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	0ccu	of Max rrence hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
2	JUNCTION JUNCTION JUNCTION	0.883	5.521	3653	22:00	266.277	7874.177
1		12.708	12.708	3623	17:00	7693.802	7693.802
5		3.416	3.416	3623	17:00	1806.085	1806.085
6	JUNCTION	1.788	5.205	3623	17:00	387.307	2193.094
WesternOutfall	OUTFALL	0.000	5.520	3653	22:00	0.000	7874.302
EasternOutfall	OUTFALL	0.000	5.192	3623	17:00	0.000	2193.124

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Node Surcharge Summary *********

Surcharging occurs when water rises above the top of the highest conduit.

Node	Туре	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2	JUNCTION JUNCTION	9.21 10.44	0.000	0.000

Node Flooding Summary

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m3
1	10.44	8.038	3623 17:00	95.291	0.000

Outfall Node	Flow	Avg.	Max.	Total
	Freq.	Flow	Flow	Volume
	Pcnt.	CMS	CMS	10^6 ltr
WesternOutfall	28.06	0.051	5.520	7874.302
EasternOutfall	20.36	0.020	5.192	2193.124
System	24.21	0.071	10.512	10067.425

Link	Type	Maximum Flow CMS	0ccu	of Max rrence hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2	CHANNEL	5.520	3653	22:00	2.81	0.91	0.92
Eastern1	CHANNEL	3.418	3623	17:00	7.16	0.06	0.24
Eastern2	CHANNEL	5.192	3623	17:00	8.04	0.09	0.30
Western1	CHANNEL	4.766	5870	12:51	2.38	1.03	1.00

Conduit		Hours Full Upstream		Hours Above Full Normal Flow	
Western1	8.92	10.34	9.21	10.49	10.34

Analysis begun on: Wed Feb 18 17:24:27 2015 Analysis ended on: Wed Feb 18 17:32:12 2015

Total elapsed time: 00:07:45

Attachment D- Hydrologic Model (Post Development LID Control)

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units CMS Process Models: Rainfall/Runoff YES Snowmelt NO ${\tt Groundwater}~\dots {\tt NO}$ Flow Routing YES Ponding Allowed NO

Water Quality NO
Infiltration Method GREEN_AMPT Flow Routing Method KINWAVE

Starting Date AUG-01-1996 00:00:00 Ending Date JAN-01-2014 06:00:00

Antecedent Dry Days 0.0 Report Time Step 00:15:00 Wet Time Step 00:15:00

Dry Time Step 01:00:00
Routing Time Step 30.00 sec

Element Count

Number of rain gages 1 Number of subcatchments ... 11 Number of nodes 6 Number of links 4 Number of pollutants 0
Number of land uses 0

****** Raingage Summary

Data Recording
Type Interval Data Source

C:\Users\Ashraf\Desktop\CLC\Climate data\RainfallData_101ROPEC_24Sept14.dat

****** Subcatchment Summary ************

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet	
EXT1	14.38	2000.00	20.00	2.0000	1	 1	
EXT2	2.95	2000.00	50.00	2.0000	1	EXW1	
BRSWM1	10.56	2000.00	50.00	2.0000	1	EXW1	
BRSWM2	6.10	2000.00	74.00	2.0000	1	EXW1	
BRSWM3	15.98	2000.00	54.00	2.0000	1	BRSWM2	
EXT3	18.39	2043.00	20.00	2.0000	1	EXW1	
EXTW	18.41	1473.00	10.00	2.0000	1	2	
EXN	36.34	3028.00	35.40	2.0000	1	5	
EXTN	23.82	2382.00	10.00	2.0000	1	6	
EXW1	74.10	3920.00	51.50	2.0000	1	1	
LIDPilot	11.60	500.00	39.00	2.0000	1	1	

****** LID Control Summary

Subcatchment	LID Control	No. of Units	Unit Area	Unit Width	% Area Covered	% Imperv Treated
LIDPilot	Bioswale	5	250.00	6.00	1.08	15.00
LIDPilot	VegetatedSwale	5	250.00	6.00	1.08	15.00

Node Summary

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Node Summary								
Name	T	Ype	Inver Elev		ax. P pth	onded Area	External Inflow	
 2		UNCTION	0.3	0 0	 .70	0.0		
1		UNCTION	1.0	0 0	.70	0.0		
5 6		UNCTION	5.0 3.0		.30	0.0		
WesternOutfal	_	UTFALL	0.0		.70	0.0		
EasternOutfal	1 0	UTFALL	1.0	0 1	.30	0.0		

Link Summary								
Name	From	Node	To Node	Туре		Length	%Slope	Roughness
 Western2	2		 WesternOutfal	1 CONDU	 тт	100.0	0.3000	0.0100
Eastern1	5		6	CONDU		50.0		0.0100
Eastern2	6 1		EasternOutfal			50.0		0.0100
Westernl	1		2	CONDU	11	400.0	0.1750	0.0100
*****	*****							
Cross Section								
Conduit	Shap	pe	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
Western2	West	ern2	0.70	2.35	0.32	7.00	1	6.07
Eastern1		ern1	1.30	4.00	0.59	6.00	1	56.19
Eastern2 Western1		ern2 ern1	1.30 0.70	4.00 2.35	0.59	6.00 7.00	1 1	56.19 4.64
Transect Summ *******								
Transect West Area:	ern1							
	.0008	0.0033	0.0075	0.0133	0.020			
	.0300	0.0409 0.1057	0.0529 0.1200	0.0654 0.1347	0.078 0.149			
0	.1655	0.1816	0.1981	0.2151	0.232	:5		
	.2504	0.2687 0.3668	0.2874 0.3878	0.3066 0.4092	0.326 0.431			
	.4534	0.4761	0.4993	0.5230	0.547			
	.5716	0.5966	0.6220	0.6478	0.674			
	.7009 .8488	0.7281 0.8837	0.7558 0.9205	0.7848 0.9593	0.815 1.000			
Hrad:								
	.0214	0.0428 0.1499	0.0643 0.1840	0.0857 0.2191	0.107			
	.2862	0.1499	0.1840	0.2191	0.410			
0	.4400	0.4689	0.4973	0.5253	0.552	19		
	.5801 .7111	0.6069 0.7365	0.6334 0.7617	0.6596 0.7867	0.685 0.811			
	.8360	0.7363	0.8845	0.7867	0.932			
0	.9563	0.9799	1.0034	1.0268	1.050	0		
	.0732	1.0962 1.1093	1.1156 1.1090	1.1135 1.1093	1.111			
Width:								
	.0400	0.0800	0.1200	0.1600	0.200			
	.2400	0.2800 0.3375	0.2949 0.3482	0.3055 0.3589	0.316			
0	.3802	0.3909	0.4015	0.4122	0.422	19		
	.4335	0.4442 0.4975	0.4549 0.5082	0.4655 0.5189	0.476 0.529			
	.5402	0.4975	0.5082	0.5189	0.529			
0	.5935	0.6042	0.6149	0.6255	0.636	2		
	.6469 .8133	0.6575 0.8600	0.6733 0.9067	0.7200 0.9533	0.766 1.000			
Transect West								
Area:	.0008	0.0033	0.0075	0.0133	0.020	9		
	.0300	0.0409	0.0529	0.0654	0.078			
	.0918 .1655	0.1057 0.1816	0.1200 0.1981	0.1347 0.2151	0.149 0.232			
WMM 5		0.1010	0.1701	·	0.232			

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Hrad:	0.2504 0.3463 0.4534 0.5716 0.7009 0.8488	0.2687 0.3668 0.4761 0.5966 0.7281 0.8837	0.2874 0.3878 0.4993 0.6220 0.7558 0.9205	0.3066 0.4092 0.5230 0.6478 0.7848 0.9593	0.3262 0.4311 0.5471 0.6742 0.8158 1.0000
Width:	0.0214 0.1285 0.2862 0.4400 0.5801 0.7111 0.8360 0.9563 1.0732 1.1103	0.0428 0.1499 0.3183 0.4689 0.6069 0.7365 0.8603 0.9799 1.0962 1.1093	0.0643 0.1840 0.3497 0.4973 0.6334 0.7617 0.8845 1.0034 1.1156 1.1090	0.0857 0.2191 0.3804 0.5253 0.6596 0.7867 0.9086 1.0268 1.1135 1.1093	0.1071 0.2531 0.4105 0.5529 0.6855 0.8114 0.9325 1.0500 1.1117 1.0000
widen.	0.0400 0.2400 0.3269 0.3802 0.4335 0.4869 0.5402 0.5935 0.6469 0.8133	0.0800 0.2800 0.3375 0.3909 0.4442 0.4975 0.5509 0.6042 0.6575 0.8600	0.1200 0.2949 0.3482 0.4015 0.4549 0.5082 0.5615 0.6149 0.6733 0.9067	0.1600 0.3055 0.3589 0.4122 0.4655 0.5189 0.5722 0.6255 0.7200 0.9533	0.2000 0.3162 0.3695 0.4229 0.4762 0.5295 0.5829 0.6362 0.7667 1.0000
Transect Area:	Easternl				
Hrad:	0.0068 0.0491 0.1056 0.1761 0.2607 0.3591 0.4697 0.5923 0.7271 0.8739	0.0141 0.0593 0.1186 0.1919 0.2793 0.3802 0.4932 0.6183 0.7555 0.9047	0.0220 0.0700 0.1321 0.2083 0.2985 0.4019 0.5173 0.6448 0.7843 0.9360	0.0305 0.0813 0.1462 0.2252 0.3182 0.4240 0.5418 0.6717 0.8137 0.9677	0.0395 0.0932 0.1609 0.2427 0.3384 0.4466 0.5668 0.6991 0.8435 1.0000
пгац.	0.0417	0.0796	0.1145	0.1472	0.1779
Width:	0.2072 0.3387 0.4558 0.5656 0.6761 0.7853 0.8908 0.9934 1.0939	0.2353 0.3629 0.4782 0.5870 0.6983 0.8067 0.9115 1.0137	0.2623 0.3867 0.5004 0.6082 0.7203 0.8279 0.9321 1.0338 1.1337	0.2884 0.4101 0.5223 0.6310 0.7422 0.8490 0.9526 1.0539 1.1535	0.3139 0.4331 0.5440 0.6536 0.7638 0.8699 0.9731 1.0740
widdii.	0.1811	0.1956	0.2100	0.2244	0.2389
	0.2533 0.3256 0.3978 0.4700 0.5362 0.5981 0.6600 0.7219 0.7838	0.2678 0.3400 0.4122 0.4844 0.5486 0.6105 0.6724 0.7343 0.7962	0.2822 0.3544 0.4267 0.4989 0.5610 0.6229 0.6848 0.7467 0.8086	0.2967 0.3689 0.4411 0.5114 0.5733 0.6352 0.6971 0.7590 0.8210	0.3111 0.3833 0.4556 0.5238 0.5857 0.6476 0.7095 0.7714 1.0000
Transect	Eastern2				
Area:	0.0068 0.0491 0.1056 0.1761 0.2607 0.3591 0.4697 0.5923 0.7271 0.8739	0.0141 0.0593 0.1186 0.1919 0.2793 0.3802 0.4932 0.6183 0.7555 0.9047	0.0220 0.0700 0.1321 0.2083 0.2985 0.4019 0.5173 0.6448 0.7843 0.9360	0.0305 0.0813 0.1462 0.2252 0.3182 0.4240 0.5418 0.6717 0.8137 0.9677	0.0395 0.0932 0.1609 0.2427 0.3384 0.4466 0.5668 0.6991 0.8435
Hrad:	0.0417 0.2072 0.3387 0.4558 0.5656	0.0796 0.2353 0.3629 0.4782 0.5870	0.1145 0.2623 0.3867 0.5004 0.6082	0.1472 0.2884 0.4101 0.5223 0.6310	0.1779 0.3139 0.4331 0.5440 0.6536
`					

Width: Transect Area:	0.6761 0.7853 0.8908 0.9934 1.0939 0.1811 0.2533 0.3256 0.3978 0.4700 0.5362 0.5981 0.6600 0.7219 0.7838 Western 0.0008 0.0300 0.0918 0.1655	0.6983 0.8067 0.9115 1.0137 1.1138 0.1956 0.2678 0.3400 0.4122 0.4844 0.5486 0.6105 0.6724 0.7343 0.7962	0.7203 0.8279 0.9321 1.0338 1.1337 0.2100 0.2822 0.3544 0.4267 0.4989 0.5610 0.6229 0.6848 0.7467 0.8086	0.8 0.9 1.0 1.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8	535 244 967 689 411 114 733 352 971 590 210	0.7638 0.8699 0.9731 1.0740 1.0000 0.2389 0.3111 0.3833 0.4556 0.5238 0.5857 0.6476 0.7095 0.7714 1.0000 0.0209 0.0784 0.1499 0.2325	
	0.2504 0.3463 0.4534 0.5716 0.7009	0.2687 0.3668 0.4761 0.5966 0.7281	0.2874 0.3878 0.4993 0.6220 0.7558	0.4 0.5 0.6 0.7	066 092 230 478 848	0.3262 0.4311 0.5471 0.6742 0.8158	
Hrad:	0.8488 0.0214 0.1285 0.2862 0.4400 0.5801 0.7111 0.8360 0.9563 1.0732 1.1103	0.8837 0.0428 0.1499 0.3183 0.4689 0.6069 0.7365 0.8603 0.9799 1.0962 1.1093	0.9205 0.0643 0.1840 0.3497 0.6334 0.7617 0.8845 1.0034 1.1156 1.1090	0.0 0.2 0.3 0.5 0.6 0.7 0.9 1.0		1.0000 0.1071 0.2531 0.4105 0.5529 0.6855 0.8114 0.9325 1.0500 1.1117 1.0000	
Width:	0.0400 0.2400 0.3269 0.3802 0.4335 0.4869 0.5402 0.5935 0.6469 0.8133	0.0800 0.2800 0.3375 0.3909 0.4442 0.4975 0.5509 0.6042 0.6575 0.8600	0.1200 0.2949 0.3482 0.4015 0.4549 0.5082 0.5615 0.6149 0.6733 0.9067	0.3 0.3 0.4 0.4 0.5 0.5	600 055 589 122 655 189 722 255 200 533	0.2000 0.3162 0.3695 0.4229 0.4762 0.5295 0.5829 0.6362 0.7667 1.0000	
Rainfall	************ File Summary ******* First Date			_	Periods w/Precip		Periods Malfunc.
1	AUG-08-199	6 DEC-31	-2013	 60 min	28488	0	0
Runoff Qu ******** Initial L Total Pre Evaporati Infiltrat Surface R Final Sur Continuit ******* Flow Rout ******** Dry Weath	********** antity Conti ******** ID Storage . cipitation . on Loss ion Loss inface Storage y Error (%) ********* ing Continui ********* er Inflow er Inflow	nuity **** **** ty ****	Volume hectare-m 0.005 3595.217 202.802 2426.025 981.441 0.011 -0.419 Volume hectare-m 0.000 981.441	104 42	Depth mm 0.020 54.656 71.779 28.685 18.891 0.046 Volume ^6 ltr 0.000 14.507		
Groundwat RDII Infl	er Inflow er Inflow ow Inflow		981.441 0.000 0.000 0.000	98	14.507 0.000 0.000 0.000		

External Outflow	972.715	9727.252
Internal Outflow	9.645	96.450
Storage Losses	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.001
Continuity Error (%)	-0.094	

All links are stable.

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 1.01

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	15454.66	0.00	423.97	12215.04	2853.71	410.37	1.14	0.185
EXT2	15454.66	0.00	978.90	7619.12	6939.50	204.72	0.28	0.449
BRSWM1	15454.66	0.00	1059.20	7627.17	6853.37	723.72	1.00	0.443
BRSWM2	15454.66	19250.81	1707.07	8318.19	24837.39	1515.09	2.10	0.716
BRSWM3	15454.66	0.00	1176.44	7019.39	7348.56	1174.31	1.53	0.475
EXT3	15454.66	0.00	429.51	12219.06	2843.68	522.96	1.42	0.184
EXTW	15454.66	0.00	225.91	13801.71	1446.36	266.28	0.88	0.094
EXN	15454.66	0.00	829.64	9724.21	4969.94	1806.09	3.42	0.322
EXTN	15454.66	0.00	291.86	13568.73	1625.97	387.31	1.79	0.105
EXW1	15454.66	4003.33	1239.31	9215.03	9082.42	6730.11	10.83	0.467
LIDPilot	15454.66	0.00	1071.37	12605.69	1847.52	214.31	0.85	0.120

Subcatchment	LID Control	Total Inflow mm	Evap Loss mm	Infil Loss mm	Surface Outflow mm	Drain Outflow mm	Init. Storage mm	Final Storage mm	Pcnt. Error
LIDPilot	Bioswale	43966.33	18626.92	25324.93	181.73	0.00	37.42	52.50	-0.41
LIDPilot	VegetatedSwale	43966.33	609.36	27476.93	16272.75		0.00	0.00	-0.89

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Occi	of Max urrence hr:min
2	JUNCTION	0.02	0.70	1.00	7	14:30
1	JUNCTION	0.02	0.70	1.70	7	14:23
5	JUNCTION	0.00	0.31	5.31	3623	17:00
6	JUNCTION	0.00	0.39	3.39	3623	17:00
WesternOutfall	OUTFALL	0.01	0.64	0.64	3653	22:00
EasternOutfall	OUTFALL	0.00	0.39	1.39	3623	17:00

Maximum Maximum Lateral Total
Lateral Total Time of Max Inflow Inflow

Node	Туре	Inflow CMS	Inflow CMS	Occurr days hr		Volume 10^6 ltr	Volume 10^6 ltr
2	JUNCTION	0.883	5.521	3653 2	2:00	266.277	7533.958
1	JUNCTION	12.822	12.822	3623 1	7:00	7354.793	7354.793
5	JUNCTION	3.416	3.416	3623 1	7:00	1806.085	1806.085
6	JUNCTION	1.788	5.205	3623 1	7:00	387.307	2193.094
WesternOutfall	OUTFALL	0.000	5.520	3653 2	2:00	0.000	7534.084
EasternOutfall	OUTFALL	0.000	5.192	3623 1	7:00	0.000	2193.124

Surcharging occurs when water rises above the top of the highest conduit.

 Max. Height
 Min. Depth

 Hours
 Above Crown
 Below Rim

 Node
 Type
 Surcharged
 Meters
 Meters

 2
 JUNCTION
 9.21
 0.000
 0.000

 1
 JUNCTION
 10.40
 0.000
 0.000

Node Flooding Summary

Flooding refers to all water that overflows a node, whether it ponds or not.

Outfall Node	Flow	Avg.	Max.	Total
	Freq.	Flow	Flow	Volume
	Pcnt.	CMS	CMS	10^6 ltr
WesternOutfall	27.80	0.049	5.520	7534.084
EasternOutfall	20.36	0.020	5.192	2193.124
System	24.08	0.069	10.512	9727.208

Link	Туре	Maximum Flow CMS	0ccu	of Max rrence hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2	CHANNEL	5.520	3653	22:00	2.81	0.91	0.92
Eastern1	CHANNEL	3.418	3623	17:00	7.16	0.06	0.24
Eastern2	CHANNEL	5.192	3623	17:00	8.04	0.09	0.30
Western1	CHANNEL	4.767	694	22:55	2.37	1.03	1.00

				Hours	Hours
		Hours Full		Above Full	Capacity
Conduit	Both Ends	Upstream	Dnstream	Normal Flow	Limited
Western1	8.93	10.30	9.21	10.43	10.30

Analysis ended on: Thu Feb 19 13:40:05 2015 Total elapsed time: 00:08:06

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
    ****************
Analysis Options
Flow Units ..... CMS
Process Models:
  Rainfall/Runoff ..... YES
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ...... YES Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... GREEN_AMPT
Flow Routing Method ..... KINWAVE
Starting Date ..... AUG-01-1996 00:00:00
Ending Date ..... JAN-01-2014 06:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ...... 00:15:00
Wet Time Step ..... 00:15:00
Dry Time Step ..... 01:00:00
Routing Time Step ...... 30.00 sec
******
Rainfall File Summary
*******
Station First Last Recording Periods Periods Periods ID Date Date Frequency w/Precip Missing Malfunc.
         AUG-08-1996 DEC-31-2013 60 min 28488
*******
                               Volume
                                             Depth
                             hectare-m
                                           mm
Runoff Ouantity Continuity
Initial LID Storage ... 0.019
Total Precipitation ... 3595.989
Evaporation Loss ... 216.887
Infiltration Loss ... 2940.633
Surface Runoff ... 456.481
Final Surface Storage ... 0.031
Continuity Error (%)
                               0.019 0.080
3595.989 15454.656
216.887 932.127
                                         12638.100
                                         1961.841
                                             0.131
*******
                               Volume
                                            Volume
Flow Routing Continuity
                             hectare-m
                                          10^6 ltr
                             -----
                                            0.000
Dry Weather Inflow .....
                             0.000
Wet Weather Inflow .....
                              456.481
                                          4564.860
Groundwater Inflow .....
                              0.000
                                           0.000
RDII Inflow .....
External Inflow .....
                             448.180
                                 0.000
                                              0.000
                                         4481.848
88 099
External Outflow .....
                                          88.099
0.000
0.000
Internal Outflow .....
                               8.810
                                0.000
0.000
0.000
Storage Losses .....
Initial Stored Volume ....
```

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-0.111

0.000

Final Stored Volume

Continuity Error (%)

All links are stable.

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 1.00

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	15454.66	0.00	423.97	12215.04	2853.71	410.37	1.14	0.185
EXT2	15454.66	0.00	978.90	7619.12	6939.50	204.72	0.28	0.449
BRSWM1	15454.66	0.00	1059.20	7627.17	6853.37	723.72	1.00	0.443
BRSWM2	15454.66	19250.81	1815.05	17223.55	15992.65	975.56	2.02	0.461
BRSWM3	15454.66	0.00	1176.44	7019.39	7348.56	1174.31	1.53	0.475
EXT3	15454.66	0.00	429.51	12219.06	2843.68	522.96	1.42	0.184
EXTW	15454.66	0.00	225.91	13801.71	1446.36	266.28	0.88	0.094
EXN	15454.66	0.00	1006.34	12565.80	1966.78	714.73	3.35	0.127
EXTN	15454.66	0.00	291.86	13568.73	1625.97	387.31	1.79	0.105
EXW	15454.66	2830.25	1297.57	13831.86	3249.15	2786.16	10.97	0.178

Subcatchment	LID Control	Total Inflow mm	Evap Loss mm	Infil Loss mm	Surface Outflow mm	Drain Outflow mm	Init. Storage	Final Storage mm	Po Er
EXN	Bioswale	56429.29	19857.31	36578.34	446.95	0.00	37.42	52.50	- (
EXN	VegetatedSwale	56429.29	718.10	25231.23	30756.14	0.00	0.00	0.00	- (
EXW	Bioswale	181894.96	22853.49	143355.88	17553.99	0.00	37.42	52.51	-]
EXW	VegetatedSwale	181894.96	758.14	52437.61	129195.83	0.00	0.00	0.00	- (

Node	Туре	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	0cci	of Max urrence hr:min
2	JUNCTION	0.01	0.70	1.00	7	14:41
1	JUNCTION	0.01	0.70	1.70	7	14:36
5	JUNCTION	0.00	0.31	5.31	3623	17:00
6	JUNCTION	0.00	0.39	3.39	3623	17:00
WesternOutfall	OUTFALL	0.01	0.64	0.64	3653	22:00
EasternOutfall	OUTFALL	0.00	0.39	1.39	3623	17:00

Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	0ccui	of Max rrence nr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
2 1 5 6 WesternOutfall EasternOutfall	JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL OUTFALL	0.883 12.115 3.351 1.788 0.000 0.000	5.521 12.115 3.351 5.137 5.520 5.132	3653 3623 3623 3623 3653 3653	22:00 17:00 17:00 17:00 22:00 17:00	266.277 3196.524 714.731 387.307 0.000 0.000	3380.094 3196.524 714.731 1101.718 3380.210 1101.617

Surcharging occurs when water rises above the top of the highest conduit.

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m3
1	9.14	7.434	3623 17:00	88.099	0.000

	Flow Freq.	Avg. Flow	Max. Flow	Total Volume
Outfall Node	Pcnt.	CMS	CMS	10^6 ltr
WesternOutfall EasternOutfall	26.31 20.43	0.023 0.010	5.520 5.132	3380.210 1101.617
System	23.37	0.033	10.452	4481.828

Link	Туре	Maximum Flow CMS	0ccu	of Max rrence hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2 Eastern1 Eastern2 Western1	CHANNEL	5.520	3653	22:00	2.81	0.91	0.92
	CHANNEL	3.349	3623	17:00	7.10	0.06	0.24
	CHANNEL	5.132	3623	17:00	8.00	0.09	0.30
	CHANNEL	4.765	6216	00:02	2.29	1.03	1.00

Conduit Surcharge Summary

				Hours	Hours
		Hours Full		Above Full	Capacity
Conduit	Both Ends	Upstream	Dnstream	Normal Flow	Limited
Western1	8.05	9.05	8.25	9.27	9.05

Analysis begun on: Wed Feb 18 17:47:32 2015 Analysis ended on: Wed Feb 18 17:55:47 2015 Total elapsed time: 00:08:15

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.0 (Build 5.0.022)
**************
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
Analysis Options
Flow Units ..... CMS
Process Models:
 Rainfall/Runoff ..... YES
 Snowmelt ...... NO
 Groundwater ..... NO
 Flow Routing ..... YES Ponding Allowed ..... NO
 Water Quality ..... NO
Infiltration Method ..... GREEN_AMPT
Flow Routing Method ..... KINWAVE
Starting Date ..... AUG-01-1996 00:00:00
Ending Date ..... SEP-01-1996 06:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ...... 00:15:00
Wet Time Step ..... 00:15:00
Dry Time Step ..... 01:00:00
Routing Time Step ..... 30.00 sec
```

WARNING 01: wet weather time step reduced to recording interval for Rain Gage 1

Element Count

Name	Data Source		Recording Interval
1	100year	INTENSITY	10 min.

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	14.38	2000.00	20.00	2.0000	 1	1
EXT2	2.95	2000.00	50.00	2.0000	1	EXW1
BRSWM1	10.56	2000.00	50.00	2.0000	1	EXW1
BRSWM2	6.10	2000.00	74.00	2.0000	1	EXW1
BRSWM3	15.98	2000.00	54.00	2.0000	1	BRSWM2
EXT3	18.39	2043.00	20.00	2.0000	1	EXW1
EXTW	18.41	1473.00	10.00	2.0000	1	2
EXN	36.34	3028.00	35.40	2.0000	1	5
EXTN	23.82	2382.00	10.00	2.0000	1	6
EXW1	74.10	3920.00	51.50	2.0000	1	1
LIDPilot	11.60	500.00	39.00	2.0000	1	1

Subcatchment	LID Control	No. of Units	Unit Area	Unit Width	% Area Covered	% Imperv Treated
LIDPilot	Bioswale	5	250.00	6.00	1.08	15.00
LIDPilot	VegetatedSwale	5	250.00	6.00	1.08	15.00

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
2	JUNCTION	0.30	0.70	0.0	
1	JUNCTION	1.00	0.70	0.0	
5	JUNCTION	5.00	1.30	0.0	
6	JUNCTION	3.00	1.30	0.0	
WesternOutfall	OUTFALL	0.00	0.70	0.0	
EasternOutfall	OUTFALL	1.00	1.30	0.0	

Name	From Node	To Node	Type	Length	%Slope R	oughness
Western2	2	WesternOutfall	CONDUIT	100.0	0.3000	0.0100
Eastern1	5	6	CONDUIT	50.0	4.0032	0.0100
Eastern2	6	EasternOutfall	CONDUIT	50.0	4.0032	0.0100
Western1	1	2	CONDUIT	400.0	0.1750	0.0100

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
Western2	Western2	0.70	2.35	0.32	7.00	1	6.07
Eastern1	Eastern1	1.30	4.00	0.59	6.00	1	56.19
Eastern2	Eastern2	1.30	4.00	0.59	6.00	1	56.19
Western1	Western1	0.70	2.35	0.32	7.00	1	4.64

0.2862

0.4400

Transect Western1 Area: 0.0209 0.0008 0.0033 0.0075 0.0133 0.0300 0.0409 0.0529 0.0654 0.0784 0.0918 0.1057 0.1200 0.1347 0.1499 0.1655 0.1816 0.1981 0.2151 0.2325 0.2504 0.2687 0.2874 0.3066 0.3262 0.3878 0.3463 0.3668 0.4092 0.4311 0.5471 0.4534 0.4761 0.4993 0.5230 0.5716 0.5966 0.6220 0.6478 0.6742 0.7009 0.7281 0.7558 0.7848 0.8158 0.8488 0.8837 0.9205 0.9593 1.0000 Hrad: 0.0214 0.0428 0.0643 0.0857 0.1071 0.1285 0.1499 0.1840 0.2531 0.2191

0.3183

0.4689

0.3497

0.4973

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0.3804

0.5253

0.4105 0.5529

Width:	0.5801	0.6069	0.6334	0.6596	0.6855
	0.7111	0.7365	0.7617	0.7867	0.8114
	0.8360	0.8603	0.8845	0.9086	0.9325
	0.9563	0.9799	1.0034	1.0268	1.0500
	1.0732	1.0962	1.1156	1.1135	1.1117
	1.1103	1.1093	1.1090	1.1093	1.0000
"I'dell'	0.0400	0.0800	0.1200	0.1600	0.2000
	0.2400	0.2800	0.2949	0.3055	0.3162
	0.3269	0.3375	0.3482	0.3589	0.3695
	0.3802	0.3909	0.4015	0.4122	0.4229
	0.4335	0.4442	0.4549	0.4655	0.4762
	0.4869	0.4975	0.5082	0.5189	0.5295
	0.5402	0.5509	0.5615	0.5722	0.5829
	0.5935	0.6042	0.6149	0.6255	0.6362
	0.6469	0.6575	0.6733	0.7200	0.7667
	0.8133	0.8600	0.9067	0.9533	1.0000
Transect W	estern2				
Area:	0.0008	0.0033	0.0075	0.0133	0.0209
	0.0300	0.0409	0.0529	0.0654	0.0784
	0.0918	0.1057	0.1200	0.1347	0.1499
	0.1655	0.1816	0.1981	0.2151	0.2325
	0.2504	0.2687	0.2874	0.3066	0.3262
	0.3463	0.3668	0.3878	0.4092	0.4311
	0.4534	0.4761	0.4993	0.5230	0.5471
	0.5716	0.5966	0.6220	0.6478	0.6742
	0.7009	0.7281	0.7558	0.7848	0.8158
Hrad:	0.8488	0.8837	0.9205	0.9593	1.0000
widele.	0.0214	0.0428	0.0643	0.0857	0.1071
	0.1285	0.1499	0.1840	0.2191	0.2531
	0.2862	0.3183	0.3497	0.3804	0.4105
	0.4400	0.4689	0.4973	0.5253	0.5529
	0.5801	0.6069	0.6334	0.6596	0.6855
	0.7111	0.7365	0.7617	0.7867	0.8114
	0.8360	0.8603	0.8845	0.9086	0.9325
	0.9563	0.9799	1.0034	1.0268	1.0500
	1.0732	1.0962	1.1156	1.1135	1.1117
	1.1103	1.1093	1.1090	1.1093	1.0000
Width:	0.0400 0.2400 0.3269 0.3802 0.4335 0.4869 0.5402 0.5935 0.6469 0.8133	0.0800 0.2800 0.3375 0.3909 0.4442 0.4975 0.5509 0.6042 0.6575 0.8600	0.1200 0.2949 0.3482 0.4015 0.4549 0.5082 0.5615 0.6149 0.6733 0.9067	0.1600 0.3055 0.3589 0.4122 0.4655 0.5189 0.5722 0.6255 0.7200 0.9533	0.2000 0.3162 0.3695 0.4229 0.4762 0.5295 0.5829 0.6362 0.7667
Transect Ea	astern1				
	0.0068 0.0491 0.1056 0.1761 0.2607 0.3591 0.4697 0.5923 0.7271 0.8739	0.0141 0.0593 0.1186 0.1919 0.2793 0.3802 0.4932 0.6183 0.7555 0.9047	0.0220 0.0700 0.1321 0.2083 0.2985 0.4019 0.5173 0.6448 0.7843	0.0305 0.0813 0.1462 0.2252 0.3182 0.4240 0.5418 0.6717 0.8137 0.9677	0.0395 0.0932 0.1609 0.2427 0.3384 0.4466 0.5668 0.6991 0.8435 1.0000
Hrad:	0.0417	0.0796	0.1145	0.1472	0.1779
	0.2072	0.2353	0.2623	0.2884	0.3139
	0.3387	0.3629	0.3867	0.4101	0.4331
	0.4558	0.4782	0.5004	0.5223	0.5440
	0.5656	0.5870	0.6082	0.6310	0.6536

Width:	0.6761 0.7853 0.8908 0.9934 1.0939	0.6983 0.8067 0.9115 1.0137 1.1138	0.7203 0.8279 0.9321 1.0338 1.1337	0.7422 0.8490 0.9526 1.0539 1.1535	0.7638 0.8699 0.9731 1.0740 1.0000
width.	0.1811 0.2533 0.3256 0.3978 0.4700 0.5362 0.5981 0.6600 0.7219 0.7838	0.1956 0.2678 0.3400 0.4122 0.4844 0.5486 0.6105 0.6724 0.7343 0.7962	0.2100 0.2822 0.3544 0.4267 0.4989 0.5610 0.6229 0.6848 0.7467 0.8086	0.2244 0.2967 0.3689 0.4411 0.5114 0.5733 0.6352 0.6971 0.7590 0.8210	0.2389 0.3111 0.3833 0.4556 0.5238 0.5857 0.6476 0.7095 0.7714
Transect E Area:	Eastern2				
Hrad:	0.0068 0.0491 0.1056 0.1761 0.2607 0.3591 0.4697 0.5923 0.7271 0.8739	0.0141 0.0593 0.1186 0.1919 0.2793 0.3802 0.4932 0.6183 0.7555 0.9047	0.0220 0.0700 0.1321 0.2083 0.2985 0.4019 0.5173 0.6448 0.7843 0.9360	0.0305 0.0813 0.1462 0.2252 0.3182 0.4240 0.5418 0.6717 0.8137 0.9677	0.0395 0.0932 0.1609 0.2427 0.3384 0.4466 0.5668 0.6991 0.8435 1.0000
niau.	0.0417 0.2072 0.3387 0.4558 0.5656 0.6761 0.7853 0.8908 0.9934 1.0939	0.0796 0.2353 0.3629 0.4782 0.5870 0.6983 0.8067 0.9115 1.0137 1.1138	0.1145 0.2623 0.3867 0.5004 0.6082 0.7203 0.8279 0.9321 1.0338 1.1337	0.1472 0.2884 0.4101 0.5223 0.6310 0.7422 0.8490 0.9526 1.0539 1.1535	0.1779 0.3139 0.4331 0.5440 0.6536 0.7638 0.8699 0.9731 1.0740
Width:	0.1811 0.2533 0.3256 0.3978 0.4700 0.5362 0.5981 0.6600 0.7219 0.7838	0.1956 0.2678 0.3400 0.4122 0.4844 0.5486 0.6105 0.6724 0.7343 0.7962	0.2100 0.2822 0.3544 0.4267 0.4989 0.5610 0.6229 0.6848 0.7467 0.8086	0.2244 0.2967 0.3689 0.4411 0.5114 0.5733 0.6352 0.6971 0.7590 0.8210	0.2389 0.3111 0.3833 0.4556 0.5238 0.5857 0.6476 0.7095 0.7714 1.0000
Transect W	Western				
Area:	0.0008 0.0300 0.0918 0.1655 0.2504 0.3463 0.4534 0.5716 0.7009 0.8488	0.0033 0.0409 0.1057 0.1816 0.2687 0.3668 0.4761 0.5966 0.7281	0.0075 0.0529 0.1200 0.1981 0.2874 0.3878 0.4993 0.6220 0.7558 0.9205	0.0133 0.0654 0.1347 0.2151 0.3066 0.4092 0.5230 0.6478 0.7848 0.9593	0.0209 0.0784 0.1499 0.2325 0.3262 0.4311 0.5471 0.6742 0.8158
Hrad:	0.0214 0.1285 0.2862 0.4400 0.5801 0.7111	0.0428 0.1499 0.3183 0.4689 0.6069 0.7365	0.0643 0.1840 0.3497 0.4973 0.6334 0.7617	0.0857 0.2191 0.3804 0.5253 0.6596 0.7867	0.1071 0.2531 0.4105 0.5529 0.6855 0.8114

	0.8360	0.8603	0.8845	0.9086	0.9325
	0.9563	0.9799	1.0034	1.0268	1.0500
	1.0732	1.0962	1.1156	1.1135	1.1117
**** 1.1 .	1.1103	1.1093	1.1090	1.1093	1.0000
Width:	0 0400	0 0000	0 1000	0 1600	0 2000
	0.0400 0.2400	0.0800	0.1200 0.2949	0.1600 0.3055	0.2000 0.3162
	0.3269	0.2300	0.3482	0.3589	0.3102
	0.3802	0.3373	0.4015	0.4122	0.4229
	0.4335	0.4442	0.4549	0.4655	0.4762
	0.4869	0.4975	0.5082	0.5189	0.5295
	0.5402	0.5509	0.5615	0.5722	0.5829
	0.5935	0.6042	0.6149	0.6255	0.6362
	0.6469	0.6575	0.6733	0.7200	0.7667
	0.8133	0.8600	0.9067	0.9533	1.0000
			_		
	******		Volume	Depth	
Runoff Quan	tity Contin	uity	hectare-m	mm	
	Storage		0.005	0.020	
	pitation		16.681	71.708	
	Loss		0.230	0.987	
	n Loss		6.886	29.601	
Surface Run	off		9.873	42.442	
Final Surfa	ce Storage		0.000	0.000	
Continuity	Error (%) .		-1.815		
			_	_	
	******		Volume	Volume	
	g Continuit ******		hectare-m	10 ^ 6 ltr	
	Inflow		0.000	0.000	
-	Inflow		9.873	98.734	
	Inflow		0.000	0.000	
			0.000	0.000	
	flow		0.000	0.000	
External Ou	tflow		5.682	56.826	
Internal Ou	tflow		4.200	41.996	
	ses		0.000	0.000	
	red Volume		0.000	0.000	
	d Volume		0.000	0.000	
Continuity	Error (%) .	• • • •	-0.088		
******	*****	*****	**		
	w Instabili				
*****	*****	*****	**		
All links a	re stable.				

Routing Tim	ne Step Summ ******	ary			
************ Minimum Tim		***	30.00 sec		
Average Tim		:			
Maximum Tim		:	30.00 sec		
	Steady Stat		0.00		
	rations per				
	1 1.01	E			

	t Runoff Su				
******	*****	****			

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	71.71	0.00	0.69	35.76	37.21	5.35	4.21	0.519

EXT2	71.71	0.00	0.78	20.77	51.42	1.52	1.35	0.717
BRSWM1	71.71	0.00	0.84	21.46	51.16	5.40	4.43	0.713
BRSWM2	71.71	136.98	1.16	14.51	193.77	11.82	7.27	0.929
BRSWM3	71.71	0.00	0.90	20.03	52.29	8.36	6.50	0.729
EXT3	71.71	0.00	0.69	36.38	36.36	6.69	4.93	0.507
EXTW	71.71	0.00	0.89	49.47	21.93	4.04	1.79	0.306
EXN	71.71	0.00	0.99	25.94	46.23	16.80	11.36	0.645
EXTN	71.71	0.00	0.94	37.90	34.12	8.13	4.18	0.476
EXW1	71.71	34.31	1.08	25.03	81.06	60.07	27.51	0.765
LIDPilot	71.71	0.00	1.70	34.11	37.48	4.35	1.88	0.523

| Total | Evap | Infil | Surface | Drain | Init. | Final | Potential | Potenti

Node Depth Summary

Node	Туре	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Occi	of Max irrence hr:min
2	JUNCTION	0.00	0.70	1.00	0	01:00
1	JUNCTION	0.00	0.70	1.70	0	00:57
5	JUNCTION	0.00	0.58	5.58	0	01:10
6	JUNCTION	0.00	0.67	3.67	0	01:10
WesternOutfall	OUTFALL	0.00	0.70	0.70	0	01:10
EasternOutfall	OUTFALL	0.00	0.67	1.67	0	01:10

Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr
2	JUNCTION JUNCTION JUNCTION JUNCTION	1.788	6.425	0 01:10	4.038	31.969
1		33.506	33.506	0 01:10	69.766	69.766
5		11.355	11.355	0 01:10	16.802	16.802
6		4.175	15.470	0 01:10	8.128	24.929
WesternOutfall	OUTFALL	0.000	6.220	0 01:17	0.000	31.896
EasternOutfall	OUTFALL		15.395	0 01:10	0.000	24.930

Surcharging occurs when water rises above the top of the highest conduit.

Node	Туре		Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2	JUNCTION	0.95	0.000	0.000
1	JUNCTION	0.98	0.000	0.000

Flooding refers to all water that overflows a node, whether it ponds or not.

				Total	Maximum
		Maximum	Time of Max	Flood	Ponded
	Hours	Rate	Occurrence	Volume	Volume
Node	Flooded	CMS	days hr:min	10 ^ 6 ltr	1000 m3
2	0.15	0.339	0 01:10	0.090	0.000
1	0.98	28.675	0 01:10	41.905	0.000

 Maximum
 Time of Max Occurrence (Ploc)
 Maximum Max/ Full
 Max/ Full
 Maximum Occurrence (Ploc)
 Max/ Full
 Full

| Hours | Hours | Hours | Capacity | Conduit | Both Ends | Upstream | Dnstream | Dnstream | Normal Flow | Limited | Limited | Western2 | 0.10 | 0.14 | 0.11 | 0.16 | 0.14 | Western1 | 0.93 | 0.97 | 0.95 | 0.99 | 0.97 |

Analysis begun on: Thu Feb 19 13:46:10 2015 Analysis ended on: Thu Feb 19 13:46:10 2015

Total elapsed time: < 1 sec

not just on results from each reporting time step.

Flow Units CMS
Process Models:
Rainfall/Runoff YES
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed NO

Water Quality NO
Infiltration Method GREEN_AMPT
Flow Routing Method KINWAVE

Antecedent Dry Days 0.0

Report Time Step 00:15:00

Wet Time Step 00:15:00

Dry Time Step 01:00:00

Routing Time Step 30.00 sec

WARNING 01: wet weather time step reduced to recording interval for Rain Gage 1

*******	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm

Initial LID Storage	0.019	0.080
Total Precipitation	16.685	71.708
Evaporation Loss	0.265	1.137
Infiltration Loss	7.330	31.503
Surface Runoff	9.430	40.529
Final Surface Storage	0.000	0.000
Continuity Error (%)	-1.925	

**************************************	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow Wet Weather Inflow	0.000	0.000 94.305
Groundwater Inflow RDII Inflow External Inflow	0.000 0.000 0.000	0.000 0.000 0.000
External Outflow	5.432 4.019 0.000	54.316 40.188 0.000
Initial Stored Volume Final Stored Volume Continuity Error (%)	0.000 0.000 -0.212	0.000

Minimum Time Step : 30.00 sec
Average Time Step : 30.00 sec
Maximum Time Step : 30.00 sec
Percent in Steady State : 0.00
Average Iterations per Step : 1.00

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
EXT1	71.71	0.00	0.69	35.76	37.21	5.35	4.21	0.519
EXT2	71.71	0.00	0.78	20.77	51.42	1.52	1.35	0.717
BRSWM1	71.71	0.00	0.84	21.46	51.16	5.40	4.43	0.713
BRSWM2	71.71	136.98	1.22	16.50	191.37	11.67	5.82	0.917
BRSWM3	71.71	0.00	0.90	20.03	52.29	8.36	6.50	0.729
EXT3	71.71	0.00	0.69	36.38	36.36	6.69	4.93	0.507
EXTW	71.71	0.00	0.89	49.47	21.93	4.04	1.79	0.306
EXN	71.71	0.00	1.48	28.94	43.12	15.67	7.89	0.601
EXTN	71.71	0.00	0.94	37.90	34.12	8.13	4.18	0.476
EXW	71.71	29.48	1.36	30.01	71.27	61.12	24.22	0.704

LID Performance Summary

| Total | Evap | Infil | Surface | Drain | Init. | Final | Potential | Potenti

Node	Туре	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	0cci	of Max urrence hr:min
2 1 5	JUNCTION JUNCTION JUNCTION	0.00 0.00 0.00	0.70 0.70 0.48	1.00 1.70 5.48	0 0 0	01:03 01:01 01:10
6 WesternOutfall EasternOutfall	JUNCTION OUTFALL OUTFALL	0.00 0.00 0.00	0.60 0.70 0.60	3.60 0.70 1.60	0 0 0	01:10 01:10 01:10

Node Inflow Summary

		Maximum	Maximum		Lateral	Total
		Lateral	Total	Time of Max	Inflow	Inflow
		Inflow	Inflow	Occurrence	Volume	Volume
Node	Type	CMS	CMS	days hr:min	10 ^ 6 ltr	10 ^ 6 ltr
2	JUNCTION	1.788	6.425	0 01:10	4.038	30.590

1	JUNCTION	26.108	26.108	0	01:20	66.468	66.468
5	JUNCTION	7.885	7.885	0	01:10	15.671	15.671
6	JUNCTION	4.175	12.010	0	01:10	8.128	23.799
WesternOutfall	OUTFALL	0.000	6.220	0	01:17	0.000	30.517
EasternOutfall	OUTFALL	0.000	12.026	0	01:10	0.000	23.799

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
2	JUNCTION	1.03	0.000	0.000
1	JUNCTION	1.05	0.000	0.000

Flooding refers to all water that overflows a node, whether it ponds or not.

Node	Hours Flooded	Maximum Rate CMS	Time of Max Occurrence days hr:min	Total Flood Volume 10^6 ltr	Maximum Ponded Volume 1000 m3
2	0.15 1.05	0.339 21.401	0 01:10 0 01:20	0.090 40.098	0.000

	Flow Freq.	Avg. Flow	Max. Flow	Total Volume
Outfall Node	Pcnt.	CMS	CMS	10 ^ 6 ltr
WesternOutfall EasternOutfall	1.12 0.96	1.010 0.921	6.220 12.026	30.517 23.799
System	1.04	1.932	18.128	54.316

Link	Type	Maximum Flow CMS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
Western2 Eastern1 Eastern2	CHANNEL CHANNEL CHANNEL	6.220 7.878 12.026	0 01:17 0 01:10 0 01:10	2.82 9.02 10.08	1.02 0.14 0.21	1.00 0.37 0.46
Western1	CHANNEL	4.751	0 02:07	2.29	1.02	1.00

		Hours Full		Hours Above Full	Hours Capacity
Conduit	Both Ends	Upstream	Dnstream	Normal Flow	Limited
Western2 Western1	0.10	0.14 1.04	0.11 1.03	0.16 1.08	0.14

Analysis begun on: Thu Feb 19 13:45:02 2015 Analysis ended on: Thu Feb 19 13:45:02 2015 Total elapsed time: < 1 sec

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